

Technical Report Documentation Page

1. REPORT No.

FHWA/CA/TE-98/13

2. GOVERNMENT ACCESSION No.**3. RECIPIENT'S CATALOG No.****4. TITLE AND SUBTITLE**

Effect of Ramp Type and Geometry on Accidents

5. REPORT DATE

November, 1998

6. PERFORMING ORGANIZATION**7. AUTHOR(S)**

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8. PERFORMING ORGANIZATION REPORT No.

51-366-680406 S

9. PERFORMING ORGANIZATION NAME AND ADDRESS

California Department of Transportation
Traffic Operations Program
1120 N Street
Sacramento, CA 95814

10. WORK UNIT No.**11. CONTRACT OR GRANT No.****12. SPONSORING AGENCY NAME AND ADDRESS**

California Department of Transportation
Traffic Operations Program
1120 N Street

13. TYPE OF REPORT & PERIOD COVERED**14. SPONSORING AGENCY CODE**

51-366

15. SUPPLEMENTARY NOTES**16. ABSTRACT**

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17. KEYWORDS

Ramp, Acceleration Deceleration lane, on-ramp, off-ramp, exit, entrance, diamond ramp, buttonhook ramp

18. No. OF PAGES:

126

19. DRI WEBSITE LINK

<http://www.dot.ca.gov/hq/research/researchreports/1997-2001/ramp.pdf>

20. FILE NAME

ramp.pdf

STATE OF CALIFORNIA. DEPARTMENT OF TRANSPORTATION
TECHNICAL REPORT DOCUMENTATION PAGE
 TROO03 (REV. 10/98)

1. REPORT NUMBER FHWA/CA/TE-98/13	2. GOVERNMENT ASSOCIATION NUMBER	3. RECIPIENTS CATALOG NUMBER
4. TITLE AND SUBTITLE Effect of Ramp Type and Geometry on Accidents	5. REPORT DATE November, 1998	6. PERFORMING ORGANIZATION CODE 51-366-680406 S
	8. PERFORMING ORGANIZATION REPORT NO.	
7. AUTHOR Ahmad Khorashadi	9. PERFORMING ORGANIZATION NAME AND ADDRESS California Department of Transportation Traffic Operations Program 1120 N Street Sacramento, CA 95814	
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19. SECURITY CLASSIFICATION (of this report) Unclassified	20. NUMBER OF PAGES 117	21. PRICE

**Effect of Ramp Type and Geometry
On Accidents**

November, 1998

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Executive Summary

Ramp design is influenced by many factors including: the speed, volume, and composition of traffic to be served, the standards and arrangement of the local street system including traffic control devices, topography, right of way controls, local planning, proximity of adjacent interchanges, community impact, consistency in the pattern of interchange ramps along a freeway and cost.

Of the total of 478,569 accidents occurring during 1992-94 on California State highways, approximately 15% occurred on ramps. Rural ramp accidents accounted for 2.82% of the accidents on rural highways, and urban ramp accidents accounted for 18.41% of the accidents on urban highways. The majority of ramps experienced single vehicle accidents or only one multiple-vehicle accident in the study period.

Ramp safety performances were compared with respect to a number of different types of accident rates (fatal, fatal + injury, total, wet, and dark accidents). A series of additional analyses were run where differences in mean accident rates between each pair of ramp configurations were estimated. The comparisons of accident rates were done for four groups of ramps stratified by ramp type (on/off), and urban/rural settings. Finally, accident occurrence on a sample of half diamond ramps was analyzed.

High accident ramps were summarized. Scissors ramps, rest area ramp, and slip ramp configurations were the most common among the ramp sample with high accident rates. Although, the percentage of the loop ramps with a left turn was relatively low in the sample, it comprises an overwhelming portion of loop ramp configuration (47%).

Accident rates for on-ramps were consistently lower than those for off-ramps (as summarized in tables 13 through 16). Most fatal and injury type

accidents occurred on off-ramps, while on-ramps tended to be linked with the less severe property damage only (PDO) accidents.

The independent variables, whose effects on accident frequency were most often found by other studies to be statistically significant, were: ramp AADT, mainline freeway AADT, area type (rural/urban), ramp type (off/on), ramp configuration, length of speed change lane, ramp length. The general consensus of many studies is that traffic volume is the strongest predictor of accidents on ramps. Ramp geometry in studies that developed accident prediction models found to be a much weaker variable than main line traffic volume and ramp traffic volume. The studies did not suggest that geometric variables are unimportant, rather, that they are generally improved to a point where further variance in geometric variables has little influence on accidents.

As a whole, literature review indicated that it is better to design ramps with flat horizontal curves (except in rural areas) avoiding the maximum degree of curvature, speed, and superelevation. Sharp curves at the end of the ramps and sudden changes from a straight alignment to sharp curves should be avoided. Ramp curves tend to be more critical than highway curves. Substantial downgrades leading to tight ramp curves may contribute to traffic accidents. Steeper grade (positive or negative) can be associated with increased accident rates. As the vertical grade of a highway increases, the accident involvement rate also increases (depending on the length of the grade). More conservative grade allowances of 3 or 4% for ramp down grades compared to the 8% found in the AASHTO "green book" has been suggested.

Truck loss of control on ramps is predominantly due to rollover and jackknifing. In designing horizontal curves to accommodate trucks, it is important to check for both rollover and skidding potential to determine which controls the design. Abrupt changes in compound curves lead to truck accidents. Jackknife accidents occur mainly at sites with low pavement friction during wet condition. Truck rollover accidents occur on ramps where truck speed is higher

than ramp design speed. Design of ramps must also provide for adequate width to accommodate offtracking during low-speed and high-speed. Lowered friction levels on high-speed ramps can contribute to truck accidents. Therefore, it recommends the resurfacing of ramps with high-friction overlays. Poor transitions to superelevation may contribute to truck accidents, such as rollovers and jackknifing. Regardless of the urban/rural location of the highway, trucks have more accidents on the off-ramps than on the on-ramps.

A good portion of the accidents associated with ramps occurs at the entrance or exit of ramps. While additional lanes improve the safety at entrance terminals, studies indicated no safety benefits as a result of auxiliary lanes at off-ramps compared to direct off-ramp designs. Findings from literature review indicated that off-ramps with speed-change lanes have a lower accident rate than those ramps without speed-change lanes. Deceleration lanes are generally safer than acceleration lanes, regardless of the length of the speed change lanes and the percentage of diverging and merging traffic. Truck accident studies are even more adamant about speed change lane, claiming that the current AASHTO requirements for deceleration lane lengths are not sufficient for trucks.

The most common accidents in the entrance terminal is the rear-end collisions. These accidents can be reduced by changing the merging maneuver at the entrance terminal to a less complex lane-changing maneuver through the use of a parallel acceleration lane. Other studies found that rear-end accidents on urban off-ramps of four configurations (diamond, parclo loop, free-flow loop, and outer connection ramps) were generally related to the operation of the cross-road ramp terminal, rather than to the geometric design of the ramp itself.

Introduction

1.1 General

The ramp is an element of an interchange system that is designed at the junction of two or more highways for the purpose of reducing or eliminating traffic conflicts, to improve safety, and increase traffic capacity. Crossing conflicts are eliminated by grade separations.

Ramp design is influenced by many factors including: the speed, volume, and composition of traffic to be served, the standards and arrangement of the local street system including traffic control devices, topography, right of way controls, local planning, proximity of adjacent interchanges, community impact, consistency in the pattern of interchange ramps along a freeway and cost. Consistency in the placement of interchanges is sometimes achieved at the cost of rearranging portions of the local street system. Figure 1 through 4 in Appendix A, illustrate typical local street interchanges, and typical freeway-to-freeway interchanges respectively.

1.2 Objective

This research project evaluates the effect of ramp configuration, type, and geometry on accidents. The analysis is aimed at investigating the accident rates and determining whether systematic differences exist between on-ramps and off-ramps, rural ramps and urban ramps and between ramps of different configurations. Since the different types of ramps are largely defined in terms of their geometric configuration, the analysis will begin to provide clues about the safety of certain geometric features.

First, a comprehensive literature review on this topic is conducted and discussed. Second, an overview of the accident experience of the ramps on the California State highways is given. Third, a statistical analysis of the accident rates on the ramps is presented. In all, accident data for 13,325 ramps are used

with the main objective of updating Lundy's study that was done for the Division of Highways (or the current California Department of Transportation).

1.3 Methodology

Three years of accident data on the existing ramps (13,325 of them) on the California State highways was retrieved. The ramp accident data and traffic volume data was obtained from the California Department of Transportation (Caltrans) Traffic Accident Surveillance and Analysis System (TASAS). The data consists of accident frequency, and rate on different ramp configurations, ramp traffic volume, ramp location, and information on urban/rural settings. The accident types included: Total, Fatal, Fatal + Injury, accidents on Wet road pavement, accident in Dark lighting condition, number of vehicles involved in accidents. Accident location for the purpose of this report is described in figure 10 in section 3.2 of this report. Any accident in areas 1 through 4 described in figure 10 section 3.2 is considered a ramp accident.

The TASAS database does not contain detailed information on ramp geometry, but rather just the code of the ramp configuration that best describes a given ramp. For this reason, most of the analysis concentrates on characterizing the systematic effects of ramp configuration on accident rates. The ramp configuration and corresponding diagram describing these configurations are provided in section 3.2

Several analyses were run on summary accident data from the years 1992 through 1994. The results from these analyses are organized as in the following: First, descriptive statistics for every ramp configuration are derived using the data retrieved from the TASAS database. The statistics include a series of tabulated detail data on every ramp configuration that included ramp frequency

distribution, mean accident rate, ramp type (on/off), and geographic area (rural and urban setting¹). These tables are provided in Appendix B of this report

Second, a series of ANOVA models were run to look at the systematic differences in accident rates between ramps of different design. These models can tell that a certain factor is influential. The significant differences with respect to ramp type (on/off), and urban vs. rural differences led to the decision to stratify the data based on whether a ramp was in a rural or urban region, as well as whether it was an on-ramp or an off-ramp. Then ramp safety performances were compared with respect to a number of different types of accident rates (fatal, fatal + injury, total, wet, and dark). The accident rates were analyzed using analysis of variance (ANOVA) and analysis of covariance (ANCOVA) methods.

Since previous studies indicated ramp traffic volume as a key variable affecting ramp accidents, a series of ANCOVA models were run, again with stratified data which included the measured ramp traffic volume as a covariate and ramp geographical location as a predictor variable. All of the ANOVA and ANCOVA analyses are followed by pairwise comparisons where differences in mean accident rates between each pair of ramp configurations can be estimated using a Tukey-Kramer adjustment method. Most of the analyses concentrated on characterizing the systematic effects of ramp configurations.

Accident rates are typically expressed in terms of the number of accidents per vehicle mile of travel to express the relative safety of road sections. Accident rate for a ramp is defined by Breuning et. al. (1960) as the number of accidents per million vehicles (MV) entering or leaving a ramp. This definition is shown to be a practical basis for measuring accident experience on ramps. The *average*

¹ The terms *rural* and *urban* (as viewed in Table 2) are not to be confused with *inside* and *outside cities*, since they are not necessarily the same. Urban areas are defined and approved by the Federal Highway Administration (FHWA) on the general basis of urban characteristics and do not necessarily coincide with city boundaries; all areas not classified as urban are considered rural.

Source: 1995 accident data on California State highways. California Department of Transportation

accident rate for a particular ramp configuration is the average across that given ramp configuration, rather than weighted according to their traffic volume. The assumption here is that accident rates on ramps are related to the number of vehicles entering or leaving the ramp without regard to the distance traveled on ramps.

Although fatal accident data is included in the analysis, it is difficult to draw sound conclusions based on results on this type of accident. The frequency of fatal accident occurrence on ramps of any type is small, so that the mean accident rates are highly variable. In other words, a few serious accidents can badly skew the statistics. Therefore, the focus of the interpretation of the results will be on, fatal + injury and total accident rates which are numerically more stable.

The accuracy of the accident data is subject to reporting levels of the law enforcement agencies supplying the collision reports. The department estimates that it receives collision reports for approximately 100 percent of all fatal accidents, 90 percent of all injury accidents, and 40 percent of all property-damage-Only (PDO) accidents occurring on state highways². Therefore, PDO is not the best source of accurate accident statistics. Fatal + Injury, and Total accidents are better measures for accident analysis.

In constructing the database on the geometric configuration of ramps, ramp configurations that were particularly rare were excluded, unless they had unusually high accident rates associated with them.

² 1995 accident data on California State highways. California Department of Transportation

Literature Review

2.1 General

Ramp safety elements include acceleration lanes, deceleration lanes, weave section, alignment and ramp terminals. The following literature review discusses the relationship between ramp configuration and geometry, and traffic accidents. Previously published research on this topic was reviewed and used in conjunction with this report's analysis to arrive at findings and conclusions. A literature review discussion is provided in this section.

Among the literature reviewed, most citations pertain specifically to the geometry of ramps, some discuss issues related to interchange, and other related to highway geometry in general. Certain geometric characteristics may be determined to affect accidents in the same way whether they belong to a regular highway segment or to a ramp interchange, such as horizontal curvature. Many of the geometric characteristics are unique to ramps. The literature review identified whether the featured study is referring to highway geometry or ramp/interchange geometry. While there are many ramp configurations defined in the American Association of State Highway and Transportation Officials (AASHTO), previous safety research basically has been focused on a limited number of ramp configurations.

To find information from ongoing studies on this topic, letters were sent to all the state DOTs inquiring whether the state agencies had conducted research on this topic. Four state agencies replied--all indicating that they have not conducted any recent research on this issue. However, as the draft of this report was ready for review, a new report on this topic was published by Federal Highway Administration (Bauer and Harwood, 1997). Special emphasis will be placed on the most recent published report by Bauer and Harwood for its innovative statistical modeling approach, and on an older report published by Lundy (1965) of

the California Division of Highways (currently California Department of Transportation). This report was initiated as an update of Lundy's study.

2.2 Literature Review Discussion

Maneuvering on ramps requires a vehicle to change speed and direction to varying degrees based upon the particulars of the ramp and the interchange design. The difficulty in maneuvering a vehicle along a ramp at speeds higher than what the geometric characteristics of the ramp are suited for is a main cause of truck accidents on ramps, according to Garber et al. (1992). Keller (1993), noted that interchanges present the motorist with a complex set of decisions that require quick evaluation and action. Oppenlander et. al (1970) noted that any discontinuity or change in driver, vehicle, roadway, traffic, and environmental conditions in the area of the interchange imposes greater demand on the highway drivers. **These discontinuities are:** necessity for driver decisions; changing proportion of vehicle types with different operating characteristics; roadway with varying geometric arrangements; changing concentrations of vehicles with resultant speed differentials; and varying environmental conditions within the interchange (ice on structural pavement). Cirillo (1968) quantified accidents rates by proximity to interchange to examine the impact of the discontinuities on interchanges. Cirillo tabulated the increases in accident rates as the distance to the interchange decreased on the exit and entrance sides of interchanges in both rural and urban locations.

Crashes not associated with ramp and interchange design can still happen as a result of vehicular or human factors. Non-geometric characteristics, from fixed road-side objects to interchange lighting, may be a factor associated with an increases in accidents and/or their severity. In fact, ramp geometry is found to be a much weaker variable than main line traffic volume and ramp traffic volume in accident prediction models developed by Bauer and Harwood (1997), and Cirillo (1968).

Several studies throughout the past 30 years have addressed the issue of whether the design of the ramp has an effect on the safety of the interchange. In the mid-1960s, Lundy (1965) produced a report for the California Department of Transportation (then the Division of Highways) addressing this very issue. The objective of Lundy's study was to determine which geometric features play important roles in ramp safety. The study involved 722 freeway ramps. Lundy found a correlation between accident rates and ramp configurations, ramp grade, fixed objects, speed change lane (SCL) lengths, possible safe entrance speeds at on-ramp noses, and off-ramp radius. The study was not able to find a correlation between ramp accident rates and on-ramp curvature, ramp lighting, ramp traffic volume, and magnitude of the ramp central angle. Other findings of Lundy's study will be discussed in the following sections.

A study of *freeway* accidents in the southern California region (Golob and Recker 1986) found that approximately 17% of accidents they reviewed occurred on ramps (including connectors). This number is strikingly similar to the 18% value Lundy found in his analysis 20 years earlier, but slightly higher than the three-year (1992-1994) average value of 15% determined in this study for urban and rural settings combined. Note however, that while combined urban-rural accidents on California state highway ramps amounts to 15% of highway accidents, the rural ramp accidents is only 2.82% of accidents on rural highways. The corresponding percent for urban ramps is 18.41%. Higher travel on urban roads in addition to other factors is responsible for this difference. Investigating strictly the truck accidents, Bowman and Hummer (1989) in an FHWA study found that up to 23.1% of accidents occurring on urban freeways happen at interchanges, with 5.7% of the accidents occurring on ramp itself, and the remainder on merging and exit areas.

As to the potential cause of some of the vehicle crashes at ramps and interchanges, several studies have reviewed ramp geometry attributes that may affect the safe operation of an interchange. Cirillo (1967) identifies sub-standard

design practices of urban interchanges as affecting the accident rate. Kihlberg and Tharp (1968) link highway geometry particularly gradients, curves, and structures to accident rates. Cirillo (1969) and Morgenstein et. al. (1978), find geometric design features of ramps to be a smaller effect than traffic volume as a predictor of accident occurrence. The general consensus of many studies is that traffic volume is the strongest predictor of accidents on ramps. When interchanges operate at or near capacity, the likelihood of increased speed differential between upstream freeway sections and interchange sections is high. Cirillo (1969) found no definitive correlation between capacity and safety other than the direct relationship of volume increase and accident frequency. Other factors identified by Cirillo associated with accident occurrence were the interchange spacing, where accident rates were shown to increase as interchange spacing decreased in urban areas as opposed to rural areas.

As many older interchanges reach the end of their design life, and are redesigned and/or rehabilitated, the consideration of safety improvements is crucial. The study by Harwood, and Graham (1983), documenting the evaluation of the effects of thirty seven interchange rehabilitation projects. In the following sections, findings of different literature on geometric and non-geometric variables affecting accident occurrence on ramps are discussed.

2.2.1 Ramp configuration

The question of whether a particular ramp configuration is operationally safer is addressed by many studies reviewed in this report, as well as the analyses performed in this study. Lundy (1965) found a correlation between accident rates and ramp configuration. According to his report, left side ramps, and scissors ramps have higher accident rates than others ramp configurations and their use is discouraged. Diamond ramps had the lowest rate, but these rates did not account for crossroad/ramp intersection accidents. Lundy did not find any significant design differences between accident-free and accident-prone ramps.

Section 3 and 4 of the current report provides a comprehensive pairwise comparison of ramps stratified by on/off and urban/rural settings.

Twomey's et al. (1992) report was primarily a literature review investigating interchange safety. The study concluded that cloverleaf, scissors, and left-side ramps should be avoided where possible. Collector distributor roads should be considered in high volume interchange design and especially designs where loop and cloverleaf ramps are used. Similarly, Leisch (1993) recommends that full-cloverleaf interchanges not be considered for an interchange. Hall (1993) studying the human factor impact suggests left-type ramps are prone to accidents. Hence, the study recommends a curvilinear design for the exit, rather than straight, to make drivers adjust their speed gradually before navigating the loop. Collector-distributor roads is a safety feature suggested by Twomey et al. (1993) and Hall (1993) to improve the safety and operation of cloverleaf interchanges. According to Hall (1993) study most of the accident problem with interchanges is related to the design of the inner loop ramps and outer connector ramps.

As a whole, research findings indicated that it is better to design ramps with flat horizontal curves (except in rural areas) avoiding the maximum degree of curvature, speed, and superelevation. Sharp curves at the end of the ramps and sudden changes from a straight alignment to sharp curves should be avoided.

Cirillo et. al. (1969) developed various mathematical models relating accidents to the a number of variables including geometric features. These models were developed for both ramp elements and six different ramp configurations. Of these variables, mainline traffic volume was found to be the most important predictor and accounted for as high as 84% of total variance in accidents while accident variance due to geometric factors accounted for 5 to 20 %. As noted by Cirillo, these results do not suggest that geometric variables are unimportant, rather that geometric factors are generally improved to a point where the variance in geometric which do occur have little influence on

accidents. Excluding other variables at the cost of losing minor accuracy, Cirillo et al. (1969), developed a plot comparing the relative safety of the various interchange types.

The most recent study by Bauer and Harwood (1997), however, stated that the regression models developed by Cirillo et al. (1969) did not make use of statistical models appropriate for accident analysis. The statistical models developed by Bauer and Harwood also found mainline and ramp traffic volume to be the most important predictors of accidents on ramps.

2.2.2 Ramp type (On-ramps vs. Off-ramps)

Lundy observed that accident rates of on-ramps were consistently lower than off-ramps accident rates (0.59 Acc/MV vs. 0.95 Acc/MV). This conclusion has also been confirmed by several other studies, including Cirillo (1967), and accident rate comparisons developed for individual ramp configurations in this report. Golob and Recker (1986) indicated that 61% of the accidents occurred on off-ramps. A study on truck accidents (Vallette et al. 1981) reveals that regardless of the urban/rural location of the highway, trucks have more accidents on the off-ramps than on the on-ramps. A truck accident study (Garber et al. 1992), analyzed the severity of the accidents and identifies most fatal and injury type accidents occurring on off-ramps, but on-ramps tended to be linked with the less severe PDO accidents. In short, these studies seem to suggest that not only do most ramp accidents occur on the off-ramps, but also the more serious accidents.

Some controversy however exists in regard to relative safety of on-ramps and off-ramps. For example, in a study of urban freeways in Texas by Mullins et al. (1961), on-ramps have higher accidents than off-ramps. This controversy is clarified in part by Cirillo (1968), where she provides data attributing the controversial results to the urban/rural differences influencing the accident rates. In order to further clarify this controversy, accident experience on 13, 325 ramps

stratified into four data sets based on ramp type (On/Off) and area type (urban/rural) are presented in section 3 and 4 of this report.

2.2.3 Ramp Deceleration/Acceleration Lanes

Studies by Lundy (1965), Fisher (1961), Golob and Recker (1986) suggest that a good portion of the accidents associated with ramps occur at the entrance or exit of ramps. Mullins et al. (1961) found that 23% of all through-lane accidents occur near entrance terminals. The entrance terminal accidents amount to 52% of the on-ramp accidents according to Lundy (1965). Bauer and Harwood (1997) considered the following factors as potential variables affecting accident occurrence on acceleration/deceleration lanes:

- Ramp AADT
- Mainline freeway AADT,
- Average lane width,
- Right shoulder width,
- Acceleration/Deceleration lane length
- Ramp area type (urban/rural), and
- Ramp configuration.

Not all variables however, were found to be statistically significant variables affecting accidents on speed change lanes (SCL). The key variables were ramp AADT, main line AADT, and SCL length. Higher traffic volumes were associated with higher accidents. The effect of acceleration lane length was more complicated since on one hand longer lengths provide safer traffic maneuvering and on the other hand corresponding increased length may provide longer exposure and more accidents.

In studies by Cirillo (1967, 1968, 1970), it was shown that accident rates decrease as length of weaving area, or acceleration and deceleration lane increases. The statistical model developed by Cirillo provided a relationship between the length of weaving and acceleration/deceleration lanes and traffic volume levels and percentage of merging and diverging traffic. Increases in

traffic volume was associated with an increase in accident rates. The effect of acceleration lane length on accident rate was significant when merging traffic percentage exceeded 6 percent of the main line traffic volume. As the percentage of merging traffic increases beyond this volume, the additional length of acceleration lane provides a significant reduction in accidents. The effect was not as great for deceleration lanes compared to acceleration lanes.

Human factors research (Hall 1993) associates higher accident rates with the inadequate length of acceleration and deceleration lanes, suggesting that the use of collector/distributor roads enhance safety. A study on two-lane loop ramps (Walker 1993) alludes many times to the fact that a speed transition zone design that accommodates enough distance for vehicles to change their speeds is important in the safety of the ramp. According to Lundy (1965), on-ramps with acceleration lane lengths greater than 800 feet had below average accident rates. The relationship between accident rate and acceleration lane length is demonstrated in figure 5.

Truck accident studies are even more adamant about this subject. Ervin et al. (1986) basically claims that the AASHTO "green book"³ requirements for deceleration lane lengths are not sufficient for trucks. Longer acceleration and deceleration lanes are also a recommendation by Jackson (1985) to improve truck safety on these highway segments, who also notes that a truck's acceleration is in general much slower than that of other vehicles on the roadway.

³ A Policy on Geometric Design of Highways and Streets, American Association of State Highway and Transportation Officials, Washington, D.C.

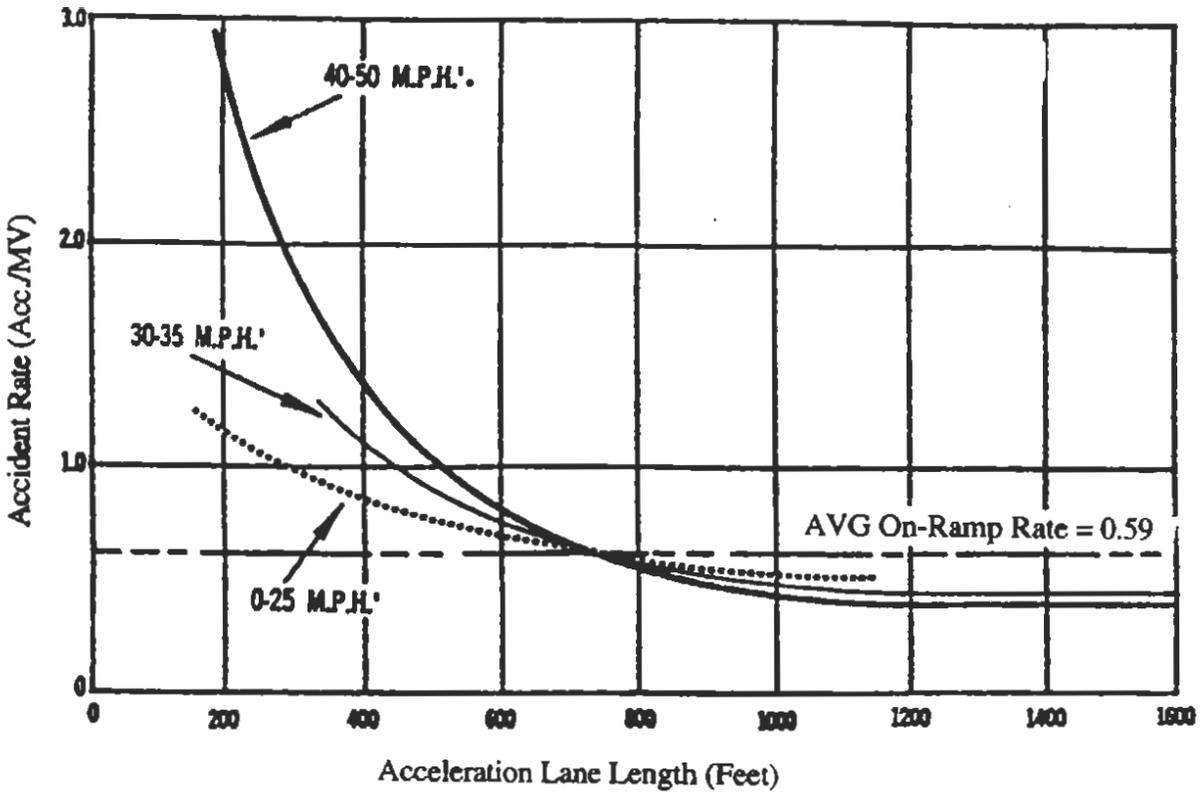


Figure 5 Accident Rate and Acceleration Lane Length (On-Ramps)

Source: "The Effect of Ramp Type and Geometry on Accidents" by R. A. Lundy, Highway Research Board Record 163, 1967, p.113

The most common accident in the entrance terminal is the rear-end collision. These accidents can be reduced by changing the merging maneuver at the entrance terminal to a less complex lane-changing maneuver through the use of a parallel acceleration lane.

Lundy (1965) found that about 44 percent of the off-ramp accidents occurred in the diverging area. Fifty percent of the off-ramp accidents are single vehicle accidents. The off-ramps with the long (900 feet +) deceleration lane lengths had lower rates than the ramps with shorter deceleration lengths. While additional lanes improves the safety at entrance terminals, Mullins et al. (1961) indicated no safety benefits as a result of auxiliary lanes at off-ramps compared to direct off-ramp designs. The study however, found off-ramps with speed-change lanes have a lower accident rate than those ramps without speed-change lanes.

Accident rates increase for increasing volume of diverging traffic regardless of the length of deceleration lane. A study by Cirillo (1969) shows very small benefits resulting from increasing the length deceleration lane when the diverging traffic is less than 6% of the main line traffic volume. Short deceleration lanes preceding tight-radius exits are found to contribute to truck accidents, according to Firestine et al. (1989).

Finally, Cirillo (1970) concludes that deceleration lane are generally safer than acceleration lanes, regardless of the length of the speed change lanes and the percentage of diverging and merging traffic.

2.2.4 Ramp grade.

Generally, the studies attempting to link vertical grades of ramps with accident rates suggest that a steeper grade (positive or negative) can be associated with increased accident rates. Miaou et al. (1993-1, 1993-2), used a Poisson regression model to analyze truck accident data on highways, found that as the vertical grade of a highway increases, the accident-involvement rate also increases (depending on the length of the grade).

Lundy found ramps associated with an overcrossing had slightly lower rates than those of undercrossings, especially the off-ramps. This may be attributed to better speed reduction on upgrade vs. down grade. Based on Lundy's report, trumpet ramps, cloverleaf ramps without collector-distributor roads and left side ramps have consistently higher accident rates than their ramps regardless of grade situations. Ervin et al. (1986), suggests more conservative grade allowances of 3 or 4% for ramp down grades compared to the 8% found in the AASHTO "green book". This would support the Firestine et al. (1989) conclusion that substantial downgrades leading to tight ramp curves contribute to traffic accidents.

2.2.5 Ramp Curvature/Radius

Ramp curves tend to be more critical than highway curves. In terms of the curvature of the ramps, Lundy (1965) makes only general observations stating a higher accident rate for ramps that are designated as "curved" than on those which were considered "straight." The straight ramps according to Lundy have lower accident rates than the curved classifications. However, he noted that the short radius, large central angle curved off-ramps, seemed to have lower rates than the ramps with medium range radii and deltas. He postulated that this was perhaps a case where the tight turns appear as obvious to the drivers and they take the necessary precautions; whereas, the medium range curves do not appear dangerous and the drivers do not compensate. Fisher (1961) studying the on-ramps without acceleration lanes, indicated that increasing ramp radius provided little benefits in term of safety and that the important factor was whether or not traffic slows down or stops before entering the freeway.

Other studies, though, analyze the curvature-accident relationship in more detail. Garber et. al. (1992), for instance, developed a linear model showing that an increase in the radius of a ramp curve produces a lower involvement of truck accidents. Likewise, Heath and Kynaston (1981) noted that many tank truck overturns occurred on ramps where the sharpness of the curve increased at the end of the ramp. The change in the curve-sharpness during the course of the ramp is pointed out as a feature to be avoided by Twomey et al. (1992). Yates (1970), investigated the accident rates for loop and outer connection ramps in cloverleaf interchanges. Twomey (1992) summarized the findings of the study by Yates as follows:

- except for loop ramps in rural areas, all right-hand side and outer-connection ramps showed an increases in accident rates with increasing maximum curvature;
- outer-connection ramps in urban areas tend to show increasing accident rates with increasing average daily traffic (ADT);

- straight outer-connections have lower accident rates than curved connections in urban and rural areas for all ADTs, except 0 to 499 in urban areas;
- rural loops with low curvature have higher accident rates than rural loops with high curvature, while the reverse is true for urban loops.

Table 1 and table 2 provide accident rates on outer connections and loops by curvature and ADT.

Table 1 Accident Rates on outer connections by curvature and ADT.

ADT	Urban Straight Curvature< 1 deg	Urban Curved Curvature>1 deg	Rural Straight Curvature< 1 deg	Rural Curved Curvature>1 deg
0-499	0.74	0.64		
500-1000	0.34	0.72	0.13	0.49
1001-1500	0.64	0.84	0.00	0.61
1501-2000	0.15	0.93	0.00*	0.20
>2000	0.49	0.82	0.00*	0.72
all volumes	0.44	0.81	0.05	0.56

Source Cirillo (1992)

* Less than 10 units

Table 2 Accident Rates on loops by curvature and ADT.

ADT	Urban Straight Curvature< 1 deg	Urban Curved Curvature>1 deg	Rural Straight Curvature< 1 deg	Rural Curved Curvature>1 deg
0-499	0.00*	0.84	1.00	0.26
500-1000	0.00*	0.96	0.81	0.37
1001-1500	1.32*	0.69	0.00*	0.00
1501-2000	0.00	0.72	0.00*	0.00
>2000	0.14	1.00	0.00*	0.00
all volumes	0.20	0.94	0.63	0.25

Source Cirillo (1992)

* Less than 10 units

Sharp curves at the end of the ramps and sudden change from straight alignment to sharp curves should be avoided. A decrease in sharp turning radii is also recommended by the truck accident study by Jackson (1985). Twomey (1992), found that in designing horizontal curves to accommodate trucks, it is important to check for both rollover and skidding potential to determine which

controls the design. Firestine et al. (1989) found that abrupt changes in compound curves lead to truck accidents.

Recent studies on ramp geometric design involved truck accidents. The general findings from the NCHRP Synthesis 241 (1997) study indicated that: 1) truck loss of control on ramps are predominantly due to rollover and jackknifing; 2) jackknife accidents occur mainly at sites with low pavement friction during wet condition; 3) truck rollover accidents occurs on ramps where truck speed is higher than ramp design speed. Design of ramps must also provide for adequate width to accommodate offtracking during low-speed and high-speed use as described in detail in the NCHRP Synthesis 241 (1997). Ramps are usually designed with adequate superelevation to reduce rollover during high-speed, but with consideration to sliding during slow speed or stopping under slippery road condition. Short radii curves such as those used on ramps may cause truck roll over. The above report provides the relationship between swept path width and ramp radius for low-speed offtracking analysis.

The selection of superelevation is part of the design process for horizontal curves based on AASHTO (1990) policy. The policy provides for establishing relationship among curvature, radius, superelevation, design speed, and length of superelevation transition and tabulated the maximum friction factor and minimum radii for various curves.

2.2.6 Ramp superelevation

Firestine et al. (1989) concludes that poor transitions to superelevation contribute to truck accidents, such as rollovers and jackknifing. Improved margin of safety in design, with special consideration of side friction factors, is suggested by this study. Since superelevation is, in most cases, left to the discretion of the highway design engineer, Keller (1993) suggests creating a larger margin of safety by going beyond the standard criteria when designing ramps to

better accommodate today's truck traffic. This view is also supported by Zador et al. (1985), who found that inadequate superelevation in the design of roadway curves, particularly on curves with grades, affects the number of fatal rollover incidents. The study suggests that care in the design must be taken to consider the behavior of some drivers taking a curve at a sharper angle than the actual curvature of the curve.

2.2.7 Non-geometric characteristics of ramps

Studies in the past have shown that accidents on ramps are not related only to the design geometry, but to other physical characteristics of the ramps. For example, Lundy (1965) stated that thirty two percent of the ramps investigated in his report were accident free. No design differences were noted between the accident-free ramps and ramps with accidents. Bauer and Harwood (1997) indicate that the accident free ramps range between 50 to 80% depending on configuration, type, and area type setting. A summary of the conclusions from various studies on accidents and non-geometric characteristics of highway and ramps is presented in this section.

2.2.8 Pavement surface conditions

Pavement design is also a factor that can be considered in the analysis of traffic accidents. Firestine et al. (1989) in an interchange/ramp accident study points out that lowered friction levels on high-speed ramps can contribute to truck accidents. Therefore, it recommends the resurfacing of ramps with high-friction overlays. The study does not give specific information on the overlays.

2.2.9 Ramp lighting

Lundy (1965) suggests that lighting is beneficial. He found that ramp areas, that have illumination at night, have a lower nighttime accident rate than other segments of freeways (generally non illuminated). It was not possible to separate the positive effects that lighting alone had on these lower accident rates.

Examining a slightly different take on interchange illumination is a study by Griffith (1994). The study compares accidents occurring on highway segments with interchange lighting only versus accidents occurring on continuously-lit segments (i.e., lighting beyond the interchange area). The study finds that the night/day accident ratio is 12% higher on segments with interchange lighting only. Therefore, the report concludes, illuminating urban freeway segments between interchanges could theoretically reduce night accidents.

2.2.10 Traffic Volume Effect

There is strong consensus among studies attributing both the ramp traffic volume and the through-lane traffic volume to the accident occurrence on ramps. Cirillo et. al. (1969) developed various mathematical models relating accident to the a number of variables including geometric features. Of these variables, mainline ADT was found to be the most important predictor and accounted for as high as 84% of total variance on average in accidents while accident variance due to geometric factors accounted for 5 to 20 %. In a recent study using more appropriate mathematical models of accident analysis, Bauer and Harwood (1997) concluded that among all variables including geometric variables, ramp volume and main line volume were the strongest predictors of accidents on ramps. Oppenlander (1970) concluded that it is safer to merge or diverge a given volume of vehicles from a freeway at several minor-flow ramps than at a single high-volume on-and-off ramps.

Lundy (1965) reported a decrease in ramp accident rates with an increase in daily traffic volume on the sample of ramps he studied. However, he did not attribute the decrease to the volume and hypothesized that high volume ramps were associated with better design standards.

2.2.11 Signing, striping, and signal visibility

Advisories and guidance to drivers approaching and navigating along a ramp is achieved through signs and striping. Walker (1993) noted that accidents were occurring at a particular ramp that lacked an advisory speed sign. Ervin et al. (1986) recommends proper placement of warning signs in advance of challenging curves and curves designed with a downgrade.

FitzPatrick et al. (1992) points out that many ramps with tight curves have posted speed signs that are inappropriate for trucks, not considering the dynamic differences between passenger-cars and trucks. Furthermore, ramps with the tight-flat-tight curve sequences should have special signing according to Ervin et al. (1986). No examples of the special signing are given, but the study suggests that the current signing practices warrant a careful evaluation.

Innovative signs are another feature for driver-warning that may help improve the safety of an interchange. For example, Fitz Patrick et al. (1992) suggest the implementation of active signs, which can alert drivers of their speed as they approach or negotiate a ramp. Also, Bonneson and Messer (1989) suggest marking and striping improvements to improve driver guidance for single-point urban interchanges. Of course, not only are signs important, but so is signal visibility, which is noted by Bonneson and Messer (1989).

2.2.12 Type of Collisions

Mullins et al. (1961) providing the accident breakdown on ramps reported that rear-end accidents accounted for 82% of all accidents. Lundy (1965) found that 42% of off-ramp accidents and 22% of on-ramp accidents involved fixed-objects, with guardrails and signs being the most frequent fixed objects involved in accidents. Lundy groups freeway fixed objects along ramps into four types: guardrails, light standards, signs, and piers, abutments, and bridge rails. Major improvement projects initiated by California Department of Transportation in

the years following the study cleared fixed objects that contributed to accidents on ramps and other road sections.

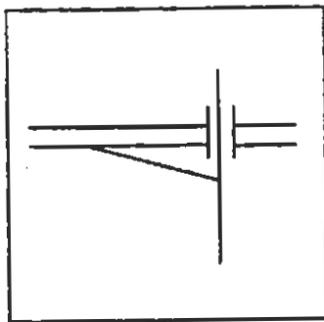
Hall and Mulinazzi (1978) include the factor of roadside objects in their analysis of highway safety. This interesting study calculated hazard indexes for 8000 combinations of speed, objects, distance, and geometric parameters for various categories of roadway volume. The research found that each type of object parameter used in the study (construction barrier, sign support, fence, curb or wall, building, guardrail, culvert or ditch, embankment, bridge, poles, light support, tree or shrubbery) appears in the top 150 hazard indices. The study also notes that objects more than 3 meters from the edge of the roadway do not appear in the top 150 indices, while most are within 1.5 meter from the road.

Some studies also focused on the effect of curbs on accidents. Two studies in particular, Ervin et al. (1986) and Firestine et al. (1989), point out that curbs on the outside of ramp curves (or, locations that have a high probability of truck rollover) is undesirable.

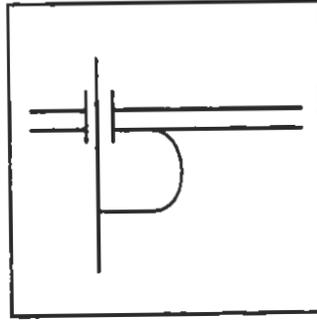
2.2.13 Statistical Modeling of Ramp Accidents

Accident prediction models developed in the past (Cirillo et. al 1969) modeled the dependent variable (Accident rate) as a function of highway -related parameters using simple multiple-linear regression. The results from these models have not been promising both in terms of the models ability to explain a proportion of the variability in accident rates and the role of geometric design variables as predictors of accidents in the models. The most recent effort in this area by Bauer and Harwood (1997) used an innovative approach to modeling accident analysis. These statistical models have greater reliability than previous work and they explained between 10 percent and 42 percent of the variability in the accident data. Developed statistical models establish relationship between traffic accidents, geometric design elements, and traffic volumes for interchange

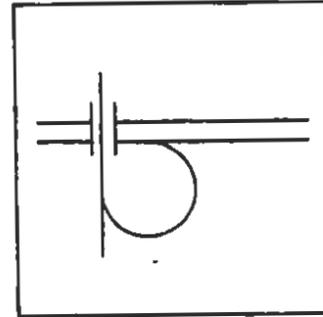
ramps and speed change lanes. Poisson and negative binomial regression models provide better representation of accident occurrences that are sporadic, random, and discrete events. The models developed were for five ramp configurations: diamond, parclo loop, free-flow loop, outer connection, direct or semi-direct connection shown below.



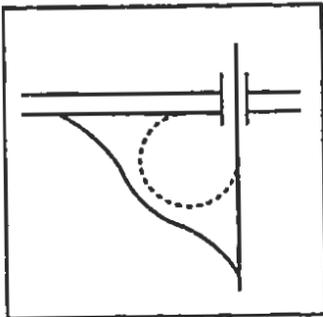
Diamond



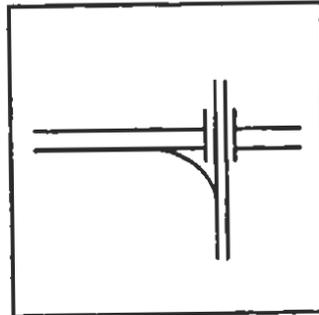
Parclo Loop



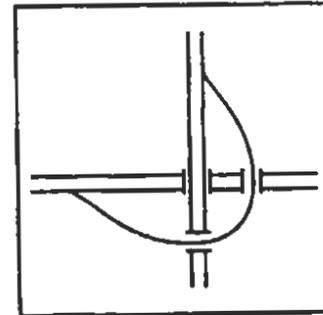
Free-Flow Loop



Outer Connection



Direct Connection^a



Semi-Direct Connection^a

Source: K.M. Bauer and D.W. Harwood (1997)

Two dependent variables most extensively used in the modeling effort were: Total multiple-vehicle accidents, and fatal plus injury multiple-vehicle accidents. Because of the limitation in availability of sufficient ramp sample sizes, statistical modeling of accidents was performed for the following combination of elements:

- Ramp proper (off- and on-ramp combined and off-ramp only)

- Entire ramp (off- and on-ramp combined and off-ramp only)
- Acceleration lanes.
- Deceleration lanes.
- Entire ramps plus adjacent speed-change lanes.
- Selected types of urban off-ramps: diamond, parclo loops, free-flow loops and outer connection ramps.

Accident prediction models for elements of ramps were initially done with the intention of combining the predicted accidents from separate models to determine the safety performance of ramps as a whole. The separate models however, by themselves, did not provide an adequate fit to the data. The best models explaining the safety performance of ramps however, were developed by combining the ramps and their adjacent speed change lane into a single model. The developed models were revised to include variables that were significant at the 20-percent significance level or better. The independent variables included in this model were:

- Ramp AADT.
- Mainline freeway AADT.
- Area type (rural/urban).
- Ramp type (off/on).
- Ramp configuration (not statistically significant for fatal + Injury Model).
- Length of SCL .
- Ramp length.

The predictive accident occurrence model for 3-year *total* multiple vehicle accident frequency for ramp, including the adjacent SCL, developed by Bauer and Harwood was:

$$Y = [e^{-7.27} (X1)^{0.78}] [e^{0.13X2}] [e^{0.45 X3}] [e^{0.78 X4}] [e^{-0.02 X5}] [e^{0.69 X6}] [e^{-0.37 X7}] [e^{0.37 X8}] [e^{-2.59 X9}] [e^{1.62X10}]$$

where:

- Y = expected number of total multiple-vehicle accidents in a 3-year period for entire ramp plus adjacent SCL
- X1 = AADT of the ramp segment (veh/day)
- X2 = mainline freeway AADT for the direction of travel in which the ramp is located (veh/day)
- X3 = 1 if the ramp is a diamond ramp; 0 otherwise
- X4 = 1 if the ramp is a parclo loop ramp; 0 otherwise
- X5 = 1 if the ramp is a free-flow loop ramp; 0 otherwise
- X6 = 1 if the ramp is a outer connection ramp; 0 otherwise
- X7 = 1 if the area type is rural; 0 otherwise
- X8 = 1 if the ramp is an off-ramp; 0 otherwise
- X9 = Speed-change lane length (ml)
- X10 = Ramp length (ml)

In the above model, the relative effect of each variable (all other variables being constant) can be calculated by simply taking the exponent of the corresponding coefficient. For example, the relative effect of the difference in accident frequency between on-ramps and off-ramps is: $e^{0.37} = 1.44$. In other word, off-ramps have more *total* multiple vehicle accidents than on-ramps by a factor of 1.44.

The expected 3-year fatal and injury multiple vehicle accident frequency for ramp, including the adjacent SCL, was estimated using the following model:

$$Y = [e^{-9.67} (X1)^{0.78}] [e^{0.23X2}] [e^{2.85 X3}] [e^{-4.42 X4}] [e^{0.48 X5}] [e^{-0.26X6}]$$

where:

- Y = expected number of fatal and injury multiple-vehicle accidents in a 3-year period for entire ramp plus adjacent SCL.
- X1 = AADT of the ramp segment (veh/day)
- X2 = mainline freeway AADT for the direction of travel in which the ramp is located (veh/day)
- X3 = ramp length (ml)
- X4 = Speed-change lane length (ml)
- X5 = 1 if the ramp is an off-ramp; 0 otherwise
- X6 = 1 if the area type is rural; 0 otherwise

Since the level of significance for the above models are set at 20 percent, caution should be exercised due to increased risk that some variables may appear to be statistically significant due to chance alone. Statistical significance is normally evaluated at 5 to 10 percent significant level.

Bauer and Harwood (1997) also modeled the expected values of accident frequency for rural off-ramps with various ramp lengths, SCL length, ramp configurations, and various levels of ramp and mainline freeway AADT. Similar information is provided for rural and urban on-ramps.

Following is a summary of the findings and conclusions derived by Bauer and Harwood (1997): Accident frequencies on interchange ramps and speed-change lanes are so low at most locations that they are very difficult to model. Between 50 percent and 80 percent of the ramps in the study experienced no multiple-vehicle accidents or only one multiple-vehicle accident in the 3-year study period; The ramp AADT was the strongest predictor of multiple-vehicle accident frequency; the other variables, while they were generally statistically significant, had much less predictive ability. This is consistent with the same conclusion drawn in virtually every study of ramp accidents. By contrast, geometric design features of ramps were found to have much less ability to predict ramp accidents; The independent variables, whose effects on accident frequency were most often found to be statistically significant, were: Ramp AADT, Mainline freeway AADT, Area type (rural/urban), Ramp type (off/on), Ramp configuration (not statistically significant for fatal + Injury Model), Length of SCL, Ramp length. Other geometric design variables considered in the modeling were Traveled-way width, right shoulder width, and left shoulder width for ramp and SCL, ramp grade, and radii of horizontal curves on ramp. None of these geometric design variables were found to have a statistically significant relationship to accident frequency, except in limited situations in models that were not ultimately recommended for use. The study indicated that rear-end accidents on urban off-ramps of diamond, parclo loop, free-flow loop, and outer

connection ramps were generally related to the operation of the cross-road ramp terminal, rather than to the geometric design of the ramp itself.

The analysis presented in the next section of this report build upon many of these conclusions by adding further insight into the potential safety issues inherent in the various designs of ramps.

Ramp Database Coding Instruction

3.1 Ramp Geometric Coding .

Figure 6 through 9 provide the ramp configuration as specified in TASAS database. A brief description of these ramp configurations is provided in Section 3.1.1.

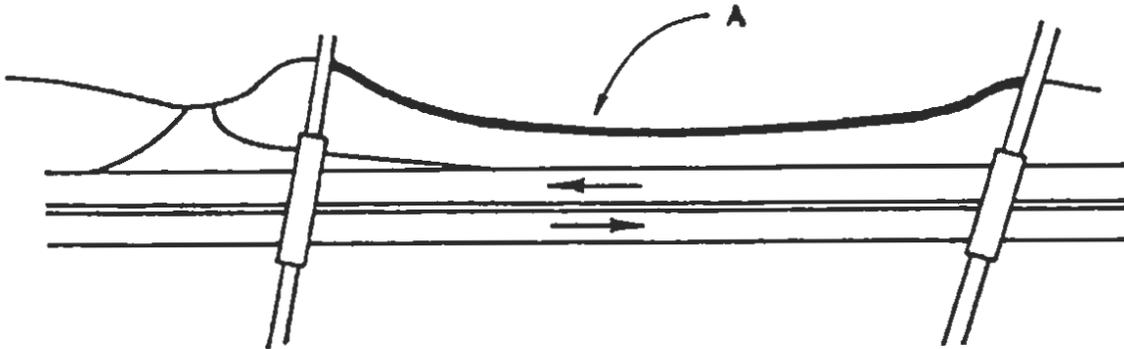


Figure 6 Frontage Roads designated as "A" in TASAS database

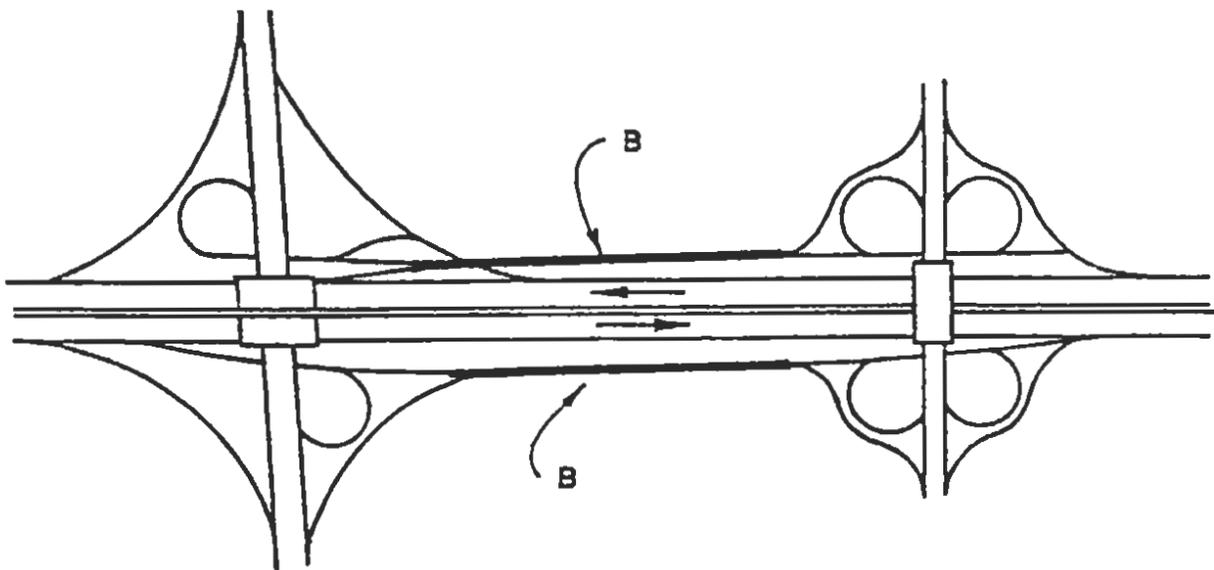
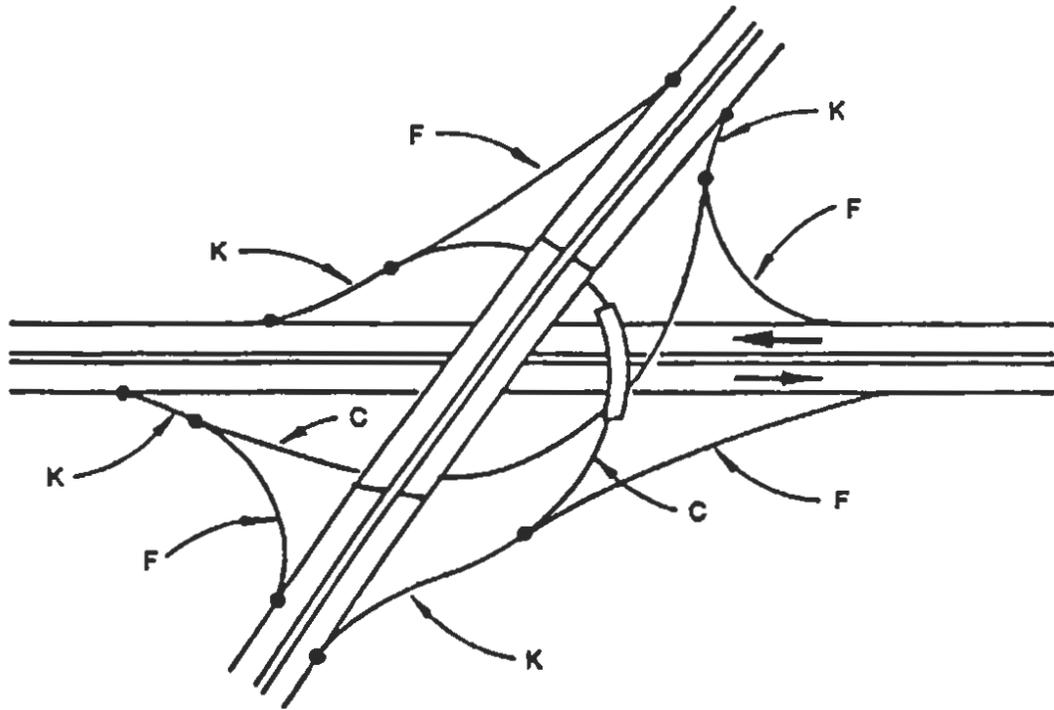
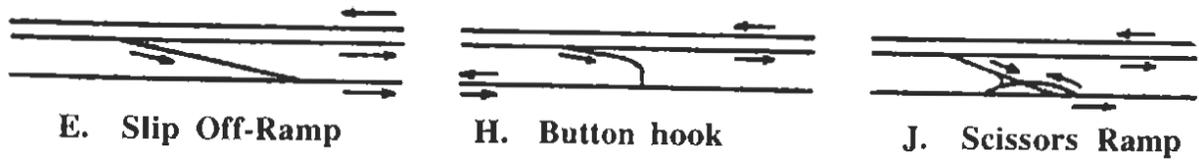


Figure 7 Collector roads designated as "B" in TASAS database



C. Direct or Semi-direct Connector
 F. Direct or Semi-direct Connector
 K. Split Ramp

D. Diamond Interchange Ramp
 G. Loop with Left Turn
 L. Loop

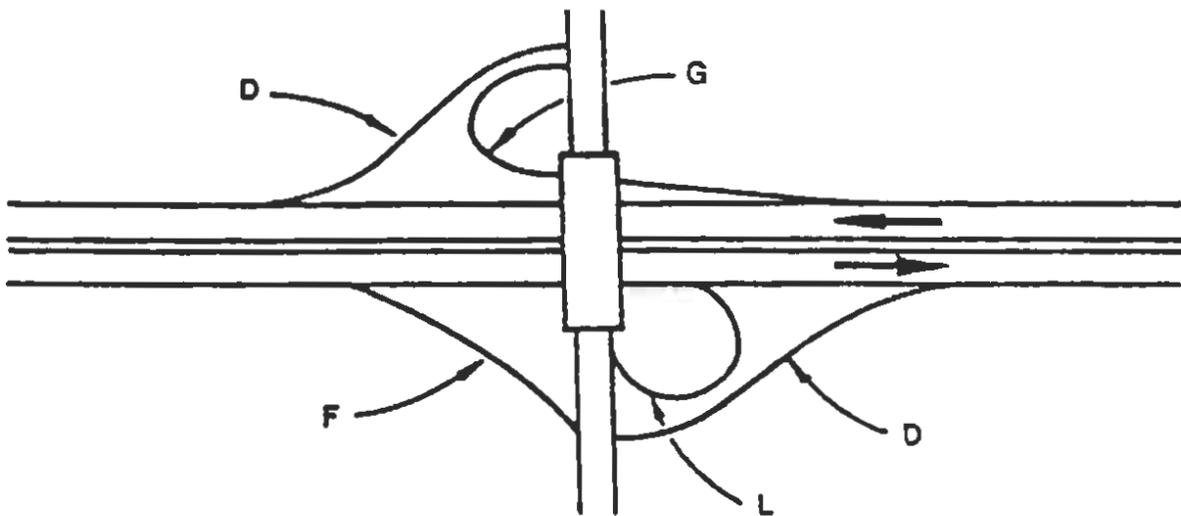


Figure 8 Ramp Configurations Designated in the TASAS data base

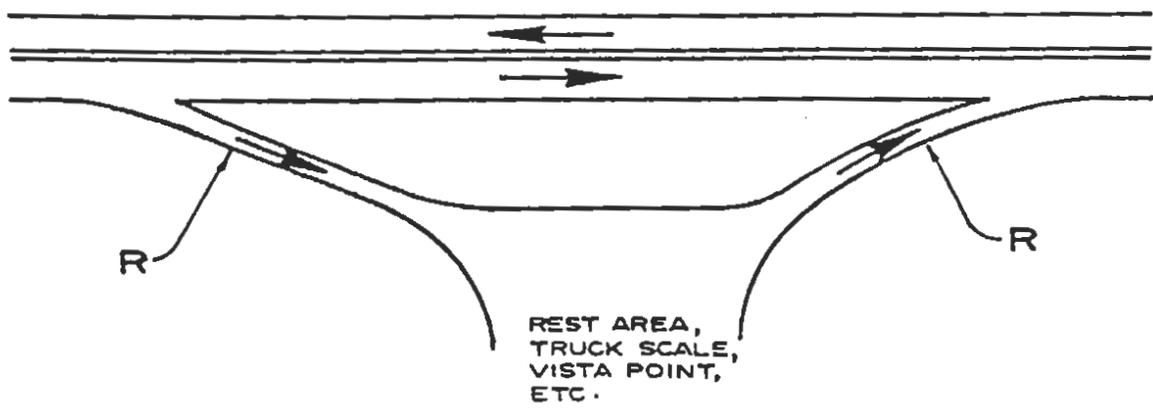
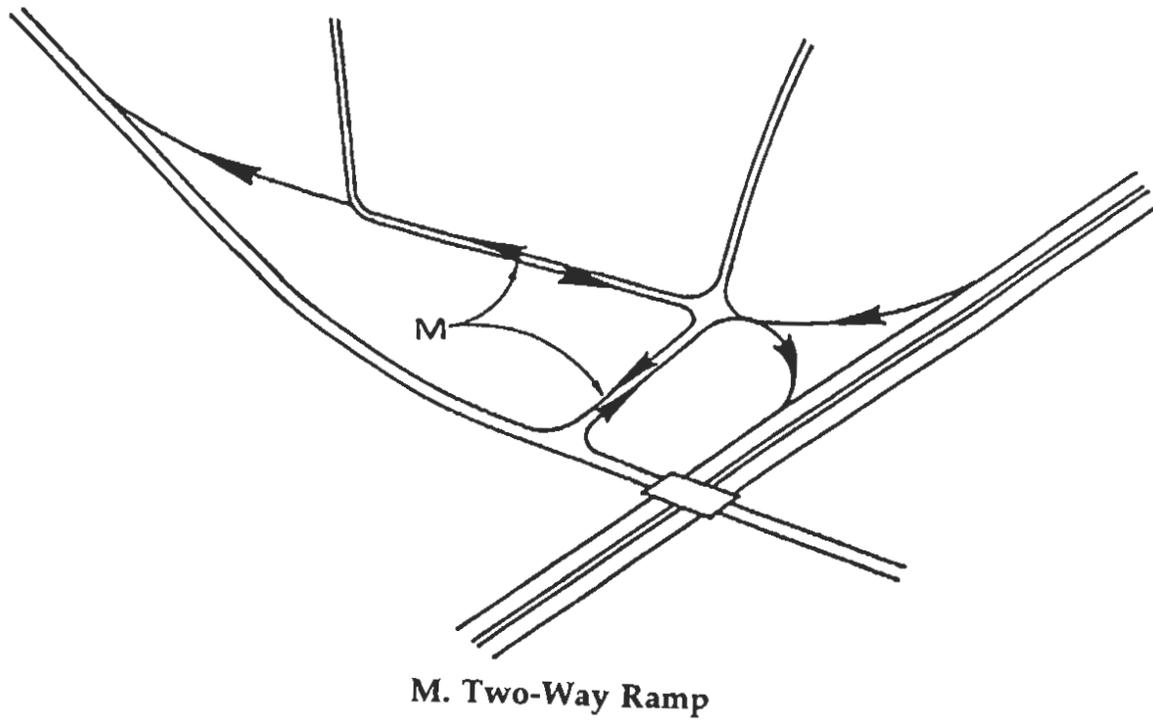


Figure 9 Ramp Configurations Designated in the TASAS data base

3.1.1 Ramp Description

The following ramp descriptions correspond to the ramp codings shown in figure 6 through 9 as specified in the TASAS database.

A. Frontage Roads

City streets or county roads generally running parallel to the freeways and providing local access.

B. Collector Roads

Separate from and generally parallel to the freeway main line and providing no local access. They carry traffic from freeway off-ramp and/or to freeway on-ramps.

C. Direct or Semi-direct Connector

Left Turning Traffic Direct connections with no left turning movements at the intersecting street terminal.

D. Diamond Interchange Ramp

Carries traffic to or from the freeway (direct-not looping movement) and there are turning and/or potential broadside accidents at the ramp terminal with the local facility.

E. Direct or Semi-direct Connector

Right Turning Traffic Direct connections with no left turning movements at the intersecting street terminal.

G. Loop with Left Turn

One-way traffic in which the turning movement is approximately 180 degrees or more. These loops also have a left-turning movement at the cross street with potential broadside accidents.

K. Split Ramp

One-way traffic from a freeway where that traffic diverges to two separate ramps or an on-ramp developed by a merge of two other ramps.

L. Loop

One-way traffic in which the turning movement is approximately 270 degrees. There may be weaving on the main line, on a collector road or on the crossing road. These loops do not have a left turning movement at the cross street.

E. Slip Ramp

Carries traffic to/from a one-way parallel facility and merges with the lane nearest the freeway of that facility and not a street crossing.

H. Button Hook

One-way ramps with a general hook maneuver approximately 90 degrees which usually terminate at streets other than the over/under crossing street.

I. Scissors

Direct one-way thru ramp traffic to or from a local two-way facility where local traffic can cross in front of the ramp traffic in generally a scissors movement which can be an angle of 90 degrees or less.

M. Two-Way Ramp

Two-way ramp segment providing no local access and where the length is so great that a separate accounting of accidents must be made for the ramp movements to or from the freeway. This element is used very seldom.

R. Rest Areas, Vista Points and Truck Scales

This ramp type used for service within the freeway Ares right of way as distinguished from interchanges which provide local ingress and egress to the freeway.

Z. All Other

All ramps that cannot be categorized in any of the above.

3.2 TASAS Accident Coding Instructions

Accident location designation on ramps are specified in the TASAS database by one of the four areas shown in figure 10. Some ramps however are specified by 3 areas. Area 4 can only be specified for a ramp interchange on a *state* road crossing a *local* road. In the case where a ramp is on a state route, any accident on a local road is specified as area 4 and as a ramp accident. When a ramp is on a state route that crosses another state route, the accident occurring in area 4 is not coded as a ramp accident rather, it is coded an accident on the state highway (i.e. the second state route). The TASAS database is set up to distinguish ramps with the above two characteristics by indicating whether the ramp has a four area accident location field or a three area accident location field.

In the remainder of this analysis, ramps with three areas are described by a Suffix "3". For instance, *loopleft3* designation refers to ramp configuration "Loop with Left Turn" for which the accidents could occur in any of the 3 areas. Similarly, *loopleft4* means that accidents on this ramp configuration are coded in areas 1 through 4. In the remainder of this report, the following ramp short designation will be used in tabulating and plotting the results:

<i>dir/semi</i>	= Direct or Semi-direct Connector
<i>loopnoleft</i>	= Loop with no Left Turn
<i>loopleft</i>	= Loop with Left Turn
<i>buttonhook</i>	= Button Hook
<i>two-way</i>	= Two-Way Ramp
<i>restarea</i>	= R. Rest Areas, Vista Points and Truck Scales

Statistical Analysis and Results

4.1 Introduction

The data analysis on ramp is organized in two parts: In part I, descriptive statistics on ramps are provided for aggregated and stratified data. In part II, the data was stratified into four subsets of data by ramp type (on/off), and ramp area (urban/rural). Then, the four data set were analyzed with respect to accident rates (fatal, fatal + injury, total, wet, and dark) using analysis of variance (ANOVA) and analysis of covariance (ANCOVA) methods. The ANOVA and ANCOVA analyses were followed by pairwise comparison of ramps. In the summary of results that are presented in the following sections, the focus of the result interpretation is fatal + injury and total accident rates as discussed before.

4.2 Descriptive Statistics on Ramps Distribution (Part I)

In aggregated level, table 3 provides distribution of ramp *type* (on/off) on California State highways. As expected, the split between on-ramps and off-ramps is just about 50/50, with two-way ramp segments only comprising 0.2% of the ramp sample.

Table 3 Off-ramp vs. On-ramp--Frequency Distribution

Ramp	Frequency	Percent
Off	6,672	50.1%
On	6,621	49.7%
Total	13,325	100.0%

Table 4 provides ramp frequency distribution by urban and rural settings.

Table 4 Location and Frequency Distribution

Ramp Location	Frequency	Percent
Rural	2,739	20.6%
Urban	10,586	79.4%
Total	13,325	100.0%

Close to 20% of ramps are in rural areas and the remaining ramps in urban areas. Table 5 provides ramp accident frequency distribution by urban and rural settings within Caltrans districts (see map in Appendix E for Caltrans districts).

Table 5 Ramp Accident Frequency by Caltrans District and Urban Rural Setting

Dist	Urban Rural	Ramp Freq	No. Accidents							MV
			Total	Fat	F+I	Mul Veh	Wet	Dark		
1	Urban	86	105	0	42	66	29	32	195	
1	Rural	243	81	1	27	47	18	15	205	
2	Urban	116	302	0	125	226	56	82	358	
2	Rural	252	99	0	34	48	21	33	214	
3	Urban	708	3541	15	1656	2689	858	911	4634	
3	Rural	458	358	6	131	199	31	100	500	
4	Urban	2541	15208	50	5768	11307	3309	4359	20581	
4	Rural	112	98	1	42	58	14	34	156	
5	Urban	400	1323	3	486	1013	175	358	1605	
5	Rural	258	257	2	86	176	22	71	373	
6	Urban	447	1555	11	646	1059	248	482	1699	
6	Rural	374	415	8	136	251	46	123	406	
7	Urban	3015	19270	87	6881	13709	2906	6694	31606	
7	Rural	142	193	1	79	90	30	67	211	
8	Urban	789	5001	23	1828	3994	623	1447	4872	
8	Rural	325	330	5	113	224	24	119	447	
9	Urban	21	23	0	12	8	1	2	22	
9	Rural	80	12	0	2	5	1	5	29	
10	Urban	554	2418	16	933	1765	446	621	2219	
10	Rural	239	334	3	128	170	41	115	372	
11	Urban	1186	5611	33	2700	4115	785	1637	11422	
11	Rural	255	185	1	47	110	16	70	321	
12	Urban	723	6654	20	2282	5178	733	2058	7753	
12	Rural	1	2	0	0	1	1	2	5	
All	Urban	10,586	61,011	258	23,35	45,129	10,169	18,683	86,968	
All	Rural	2,739	2,364	28	825	1,379	265	754	3,239	
	Total	13,325	63,375	286	24,18	46,508	10,434	19,437	90,206	

* Accidents/MV

Every ramp configuration can be located in both an urban and rural setting, with the exception of the slip-3 ramps, which are only present in urban areas. Note from Table 4 that most ramps occur in an urban setting. This is also

reflected in Table 5, where the Districts with the most ramps contain the major metropolitan areas of Los Angeles and Ventura counties (District 7), the San Francisco Bay Area (District 4), San Diego county (District 11), and the Sacramento metropolitan area (District 3). The only ramp configuration that is an exception is the rest area/truck scale/vista point ramps. The majority of this type of ramp is found in rural areas.

4.3 Ramp Accidents Frequency and Severity

Of the total of 478,569 accidents occurring during 1992-94 on California State highway ramps, approximately 15% occurred on ramps as demonstrated in table 6. However, rural ramps accounted for 2.8% of the accidents occurring on rural highways, where as the urban ramp accidents accounted for 18.41% of the accidents of the urban highways.

Table 6 Ramp vs. Highways Accidents (1992-94)

State Highway Type	Total Acc.	F+I Acc.
Rural Non-Freeway	36,012	27,600
Rural Freeway	25,733	12,345
Urban Non-Freeway	88,875	39,754
Urban Freeway	241,772	91,200
Total Rural	61,745	40,000
Total Urban	330,647	120,963
Total (Urban + Rural)	392,392	160,963
Ramps (Rural)	2,364	825
Ramps (Urban)	61,011	23,359
Ramps (Urban + Rural)	63,375	24,184
Ramp Acc. As % of	%	%
Ramps (Rural)	2.82%	2.10%
Ramps (Urban)	18.41%	19.31%
Ramps (Urban + Rural)	15.26%	15.02%

The accident frequency and severity ratio calculated based on 1992-1994 accident data on California are presented in table 7, and table 8 respectively. These severity ratios show improvement from the severity ratios measured by Lundy

(1965) where the fatal, injury and PDO accounted for 1.8, 42.9, and 55.3 percent respectively.

Table 7 Three year Accident Frequencies

Acc Type	Off-Rural	Off-Urban	On-Rural	On-Urban
Fatal	23	167	4	91
Injury	547	15115	248	7948
PDO	1058	23654	478	13931
Total	1628	38936	730	21970

1992-through 1994 accident data

Table 8 Accident Severity Ratio

Acc Type	Off-Rural	Off-Urban	On-Rural	On-Urban
Fatal	1.4 %	0.4 %	0.5 %	0.4 %
Injury	33.6 %	38.8 %	34.0 %	36.2 %
PDO	65.0 %	60.8 %	65.5 %	63.4 %

Attention should be paid to the level of aggregation of data considered in tabulating or plotting data since some level of detail is lost as a result of aggregating information. For example, in figure 11, differences in urban vs. rural and differences in individual ramp configurations are ignored in the interest of providing a comparison of on-ramps vs. off-ramps in general.

Note that off ramps are clearly more accident prone than on ramps both in terms of frequency of accident occurrence and the average accident rates. The total number of accidents on off-ramps are approximately twice the number of accidents on the on-ramps aggregating all configurations in urban and rural settings.

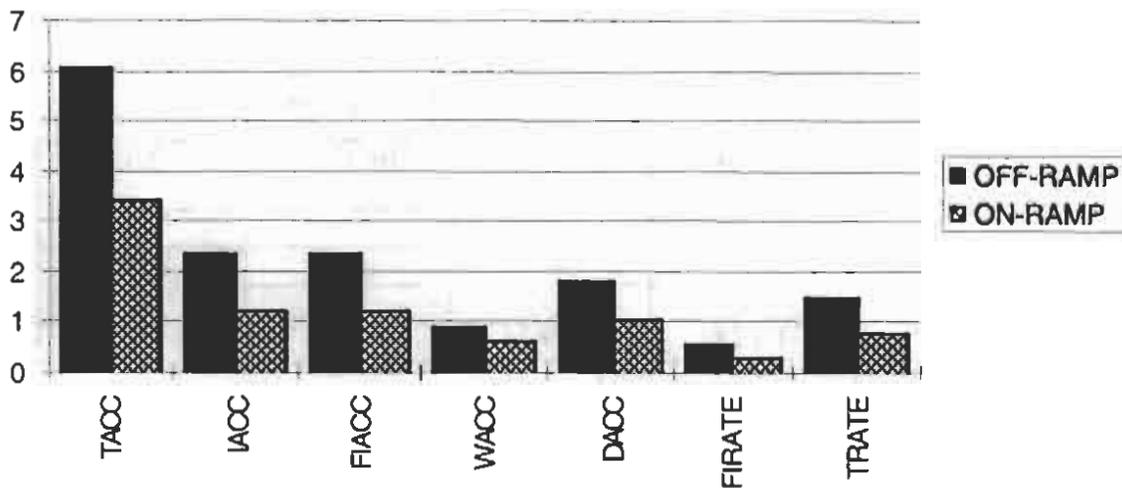


Figure 11. On-Ramp vs. Off-Ramp Average Accident Frequencies

Legends

- TACC = Total Accident,
- IACC = Injury Accident ,
- FIACC = Fatal + Injury (F+I) Accidents,
- WAOC = Accidents occurring in wet pavement surface conditions,
- DAOC = Accidents occurring during dark lighting conditions,
- TRATE = Total Accident_(No. of Accidents/MV)
- FIRATE = Fatal + Injury Accident Rate (No. of Accidents/MV)

When interpreting the *average* accident rates, the reader should take into consideration the wide *variability* in number of accidents occurring on ramps within a particular configuration. For example, while the average frequency of *total* accident for the off-ramps is 6.08 accidents, its variability ranges from a minimum of zero accidents to a maximum of 117 accidents in the three year period.

For the remaining analyses, the data is stratified into 4 groups of data:

- 1) on ramps in rural area;
- 2) on-ramps in urban area;
- 3) off-ramps in rural area;
- 4) off-ramps in urban area.

4.3 Accident Frequency and rate (Urban/Rural Differences)

Aggregating ramps of different configurations, figure 12 and 13 and corresponding tables 9 demonstrate that accident *rates* on urban ramps are lower for both on-ramps and off-ramps. When accident *frequencies* are considered however, the opposite is true, meaning that accident frequencies are significantly higher on urban ramps (see figure 14 and 15).

Figure 12 Rural vs. Urban Accident Rate, All Off-Ramps Combined

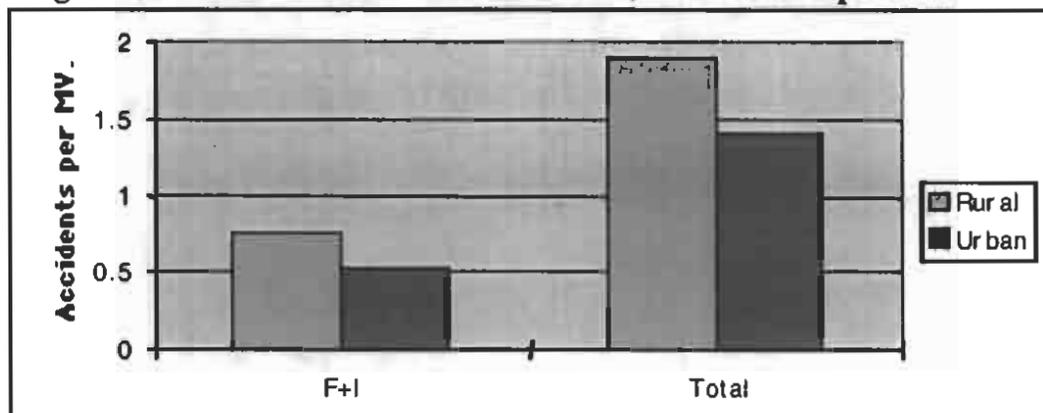


Figure 13 Rural vs. Urban Accident Rate, All On-Ramps Combined

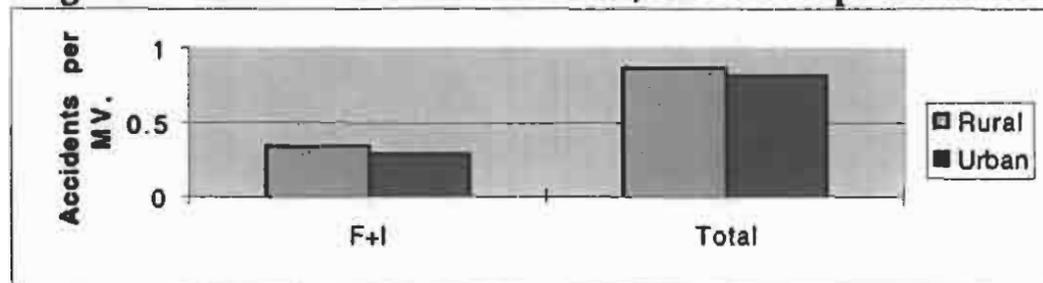


Table 9 Urban vs. Rural Ramp Accident Rate

	Accident Type	Off- Ramp				On- Ramp			
		Mean	Std Dev	Min.	Max.	Mean	Std Dev	Min.	Max.
Rural	Fatal +Injury	0.75	3.65	0.00	82.19	0.35	2.82	0.00	82.94
Rural	Total	1.87	5.63	0.00	82.19	0.88	4.13	0.00	82.94
Urban	Fatal+Injury	0.53	1.81	0.00	96.08	0.30	1.02	0.00	30.44
Urban	Total	1.39	5.34	0.00	198.39	0.81	3.77	0.00	234.45

Total number of ramps in urban area = 10,564 and in rural area =2729

Figure 14 Average Accident Frequency (Off-Ramps Combined)

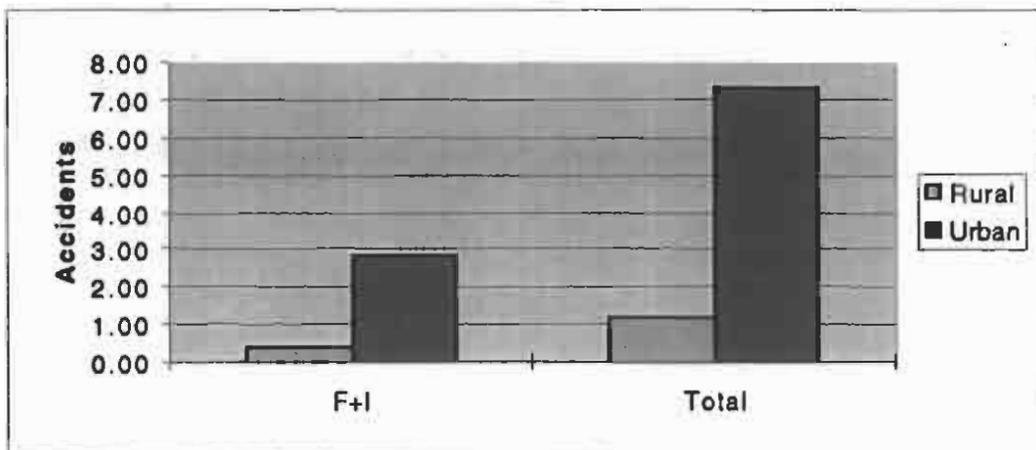
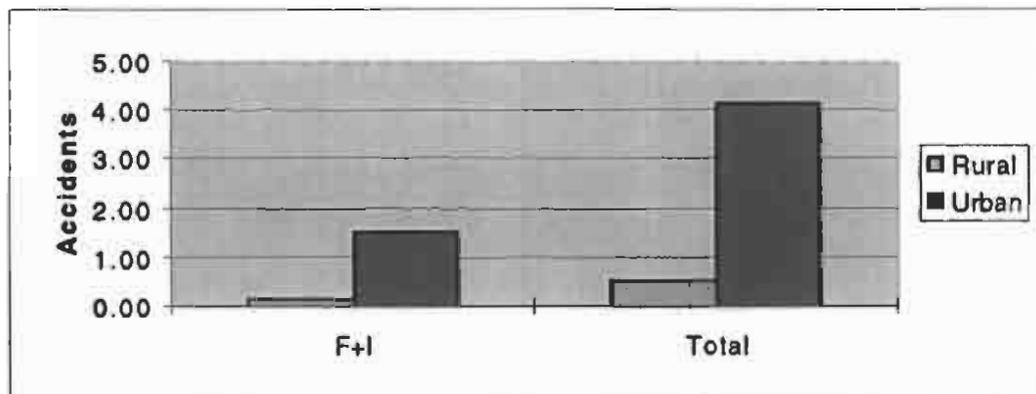


Figure 15 Average Accident Frequency (On-Ramps Combined)



4.5 Descriptive Statistics on Individual Ramp Configurations

Table 10 provides a sample of descriptive statistics on individual ramps. Complete statistics similar to the information in table 10 on 15 ramp configurations is provided in Appendix B. This includes a total of 60 tables [15x4 settings : on/off by urban/rural) All statistics are based on averaging three years of accident data (1992-1994).

**Table 10 Sample of Descriptive Statistics on *buttonhook* Ramps
(see Appendix B for other ramp configurations)**

Ramp Type= **Off** Urban/Rural =**Rural** DESCR=**buttonhook**

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC*	111	1.3513514	1.7666025	0	8.0000000
FACC*	111	0.0090090	0.0949158	0	1.0000000
IACC*	111	0.4774775	0.9327072	0	5.0000000
FIACC*	111	0.4864865	0.9328828	0	5.0000000
MVACC*	111	0.5585586	1.3258595	0	8.0000000
WACC*	111	0.0630631	0.2441787	0	1.0000000
DACC*	111	0.3963964	0.6505841	0	2.0000000
XVOL*	111	0.7531532	0.8146187	0	3.7000000
RVOL*	111	0.8185586	0.8891281	0.0200000	4.0500000
FRATE*	111	0.0433063	0.4562602	0	4.8070000
FIRATE*	111	1.6695495	8.1557984	0	82.1900000
TRATE*	111	4.1284685	11.3893260	0	82.1900000
WRATE*	111	0.1269562	0.7101832	0	6.5200000
DRATE*	111	1.3286379	5.5941815	0	54.7933333

*** Legend**

- N = Number of ramps for this configuration
- TACC = Total Accidents
- FACC = Fatal Accidents
- IACC = Injury Accidents
- FIACC = Fatal + Injury Accidents
- MVACC = Multi-vehicle Accidents
- WACC = Wet Accidents
- DACC = Dark Accidents
- XVOL = Ramp Cross Street Traffic Volume (MV)
- RVOL = Ramp Traffic Volume (MV)
- FRATE = Fatal Accident Rate (Acc/MV)
- FIRATE = Fatal + Injury Accident Rate (Acc/MV)
- TRATE = Total Accident Rate (Acc/MV)
- WRATE = Wet Accident Rate (Acc/MV)
- DRATE = Dark Accident Rate (Acc/MV)

Part II of the statistical analysis that follows provides accident rates comparison of the stratified data after adjustment is made for ramp volume, and geographical effect (Caltrans districts) is included in the ANOVA analysis as a predictor.

4.6 Statistical Analysis Part II (Stratified Data)

It is important to realize that an apparent ramp-type effect might be explainable in terms of other systematic factors, such as traffic patterns differing between rural and urban regions, differences attributed to Caltrans districts. The types of ramps that are predominantly featured can differ between urban and rural freeways or among Caltrans districts. For instance, although approximately 80% of ramps are in urban areas, nearly 85% of diamond ramps are in rural area. This could result in rural/urban differences in accident rates being falsely attributed to the types of ramps that are common in those areas.

In response to this possibility, a series of analysis of variance (ANOVA) and analysis of covariance models were run in which urban and rural differences, differences between districts, and differences between on-ramps and off-ramps were taken into account. The statistically significant differences in ramp configurations, type, and urban/rural setting resulting from these models led to the decision to split the data into four groups. The analysis in Part II dealt with analysis of each group separately.

Each separate analysis included a comparison of ramp configurations with respect to different types of accident rates (fatal, fatal + injury, total, wet, and dark). These analyses were performed to determine if in fact the apparent difference between ramps are statistically significant. In the following section only a summary of ANOVA and ANCOVA results are provided. More detail information but, still in a summarized format extracted from the statistical analyses is provided in Appendix C of this report. All ANOVA/ANCOVA analyses in Appendix C are followed by pairwise comparison tables. To present the results in a usable format for engineers, only the pairwise comparisons data that provide average accident rates are presented in the report. Complete ANOVA and ANCOVA tables can be made available upon request from the author of this report.

4.6.1 Example of Ramp Pairwise Comparison

Table 11 is presented as a sample of pairwise comparison. Notice that rarely a value of the accident mean is calculated as negative. This due to the very small number of accidents or accidents, and a higher than average ramp volume, in the subgroup and how the software program performing the LSMEAN provide relative comparison of the subgroups. In other word the program may figure that "if the ramp volume was even lower, the accidents would be negative". Of course, this makes no practical sense, but since the program knows nothing about the data being bounded below by zero, some LSMEANS are translated into negative values. In a way, the means have relative values. One option is to assume the negative means as zero and adjust the other means when comparison is made. This compensation is made in figure 16 through 19 and corresponding tables 13 through 16. As noted before, statistical analysis that follows provides accident rates comparison of the stratified data *after* adjustment is made for ramp volume, and the geographical effect (Caltrans districts) is included in the ANOVA analysis as a predictor.

Table 11 Sample of Least Squares Means for Fatal + Injury Accident Rate (see Appendix C for other cases)

Adjustment for multiple comparisons: Tukey-Kramer

DESCR**	FIRATE* LSMEAN***	Number
buttonhook	0.20219715	1
diamond3	0.17053038	2
diamond4	0.37598087	3
dir/semi3	0.24141925	4
dir/semi4	0.26064053	5
direct	0.34373484	6
loopleft3	0.30416232	7
loopleft4	0.40222718	8
loopnoleft3	0.19102482	9
loopnoleft4	0.26318905	10
restarea	-0.03051604	11
scissors	0.20071572	12
slip3	0.29200270	13
slip4	0.53264205	14
split	0.19501126	15

* FIRATE= Fatal + Injury Accident Rate

** DESCR = Ramp Description

*** LSMEAN = Least Mean Square accident rate

Table 12 provides the p-value matrix. Each entry determines whether the pairwise comparison is significant. For example, from table 11, the mean fatal + injury accident rate (FIRATE) for diamond4 and direct ramp (row 3, and 6) are 0.37598087 and 0.34373484 respectively. The entry in row 3 and column 6 of table 12 provide a p-value = 1.0 which is greater than 0.05. Therefore, the difference between the two mean accident rates is not statistically significant.

0.113
0.07
0.1234

0.05
0.076

Table 12 Matrix of Statistical Significance (p-values <0.05) for i/j Ramp Pairs

i/j	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
1	.	1.0000	0.0697	1.0000	0.9998	0.9679	1.0000	0.4036	1.0000	0.9998	0.9893	1.0000	1.0000	0.3318	1.0000
2	1.0000	.	0.7168	1.0000	0.9999	0.9851	1.0000	0.7633	1.0000	0.9999	0.9992	1.0000	1.0000	0.4823	1.0000
3	0.0697	0.7168	.	0.7514	0.3928	1.0000	1.0000	1.0000	0.6918	0.5628	0.4567	0.7992	1.0000	0.9920	0.2372
4	1.0000	1.0000	0.7514	.	1.0000	0.9992	1.0000	0.8791	1.0000	1.0000	0.9658	1.0000	1.0000	0.6242	1.0000
5	0.9998	0.9999	0.3928	1.0000	.	0.9998	1.0000	0.8373	1.0000	1.0000	0.9186	1.0000	1.0000	0.6305	0.9999
6	0.9679	0.9851	1.0000	0.9992	0.9998	.	1.0000	1.0000	0.9907	0.9999	0.7525	0.9959	1.0000	0.9896	0.9602
7	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	0.9998	1.0000	1.0000	1.0000	1.0000	1.0000
8	0.4036	0.7633	1.0000	0.8791	0.8373	1.0000	1.0000	.	0.7715	0.8797	0.4492	0.8446	1.0000	0.9997	0.5248
9	1.0000	1.0000	0.6918	1.0000	1.0000	0.9907	1.0000	0.7715	.	1.0000	0.9969	1.0000	1.0000	0.4996	1.0000
10	0.9998	0.9999	0.5628	1.0000	1.0000	0.9999	1.0000	0.8797	1.0000	.	0.9165	1.0000	1.0000	0.6640	0.9999
11	0.9893	0.9992	0.4567	0.9658	0.9186	0.7525	0.9998	0.4492	0.9969	0.9165	.	0.9954	0.9655	0.2316	0.9940
12	1.0000	1.0000	0.7992	1.0000	1.0000	0.9959	1.0000	0.8446	1.0000	1.0000	0.9954	.	1.0000	0.5686	1.0000
13	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9655	1.0000	.	0.9858	1.0000
14	0.3318	0.4823	0.9920	0.6242	0.6305	0.9896	1.0000	0.9997	0.4996	0.6640	0.2316	0.5686	0.9858	.	0.3478
15	1.0000	1.0000	0.2372	1.0000	0.9999	0.9602	1.0000	0.5248	1.0000	0.9999	0.9940	1.0000	1.0000	0.3478	.

In the absence of a statistically significant difference between a particular pair of ramp types, any of the following explanations is plausible:

- there is no difference between the ramp types;
- there is a difference, which went undetected due to the conservatism of the *post hoc* analysis;
- there is a difference, which went undetected because the magnitude of the difference was small; or
- there is a difference, which went undetected because there were relatively few ramps for the two types being compared.

In view of these four possibilities, it's appropriate that the distinction between a significant difference and an insignificant one should be interpreted as being between those differences for which there was sufficient evidence to declare a difference, and those for which there simply was insufficient evidence.

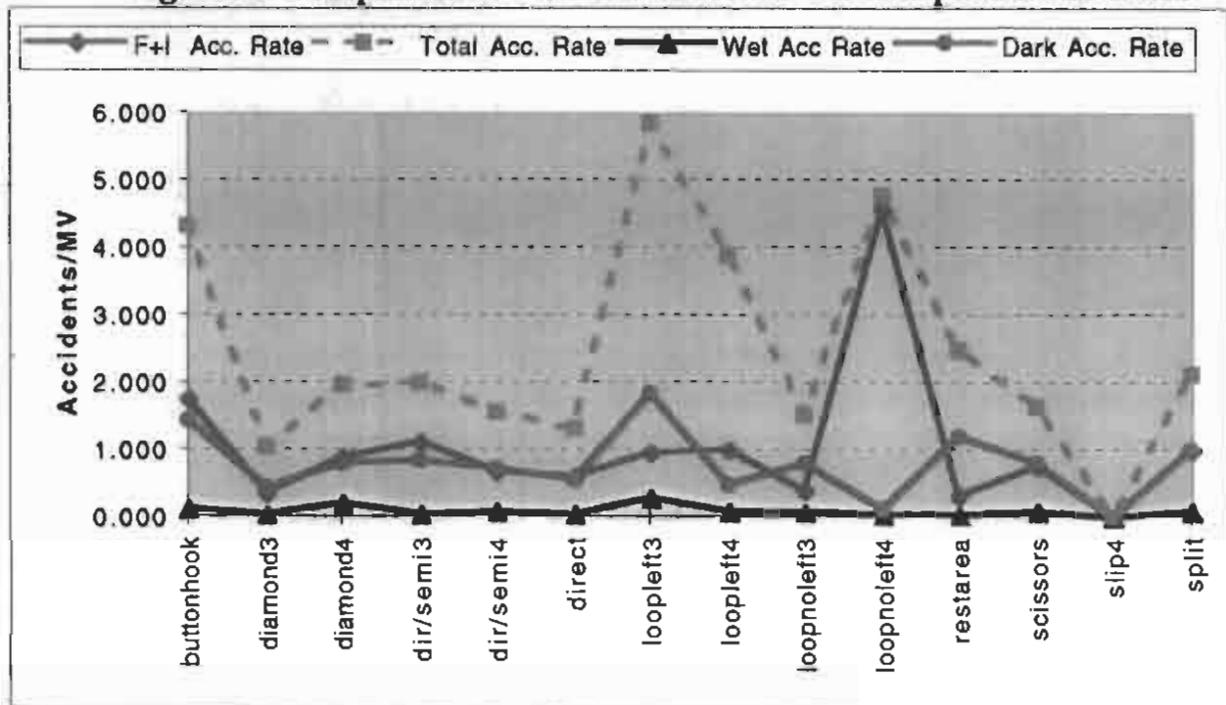
Similar tables to table 11 and 12 are presented in Appendix C, organized into four sets of tables as follows:

- 1) Analysis of Off-ramps in rural area
- 2) Analysis of Off-ramps in urban area
- 3) Analysis of On-ramps in rural area
- 4) Analysis of On-ramps in urban area

For each of the above analyses, a summarized ANOVA table is first presented. These tables provide information as to whether the factors in the model are significant or not. These tables are then followed by pairwise comparison similar to table 11 and 12 where average accident rates for different ramp configurations are compared. The matrix of p-values similar to table 12 indicate whether the difference in mean accident rates are significant.

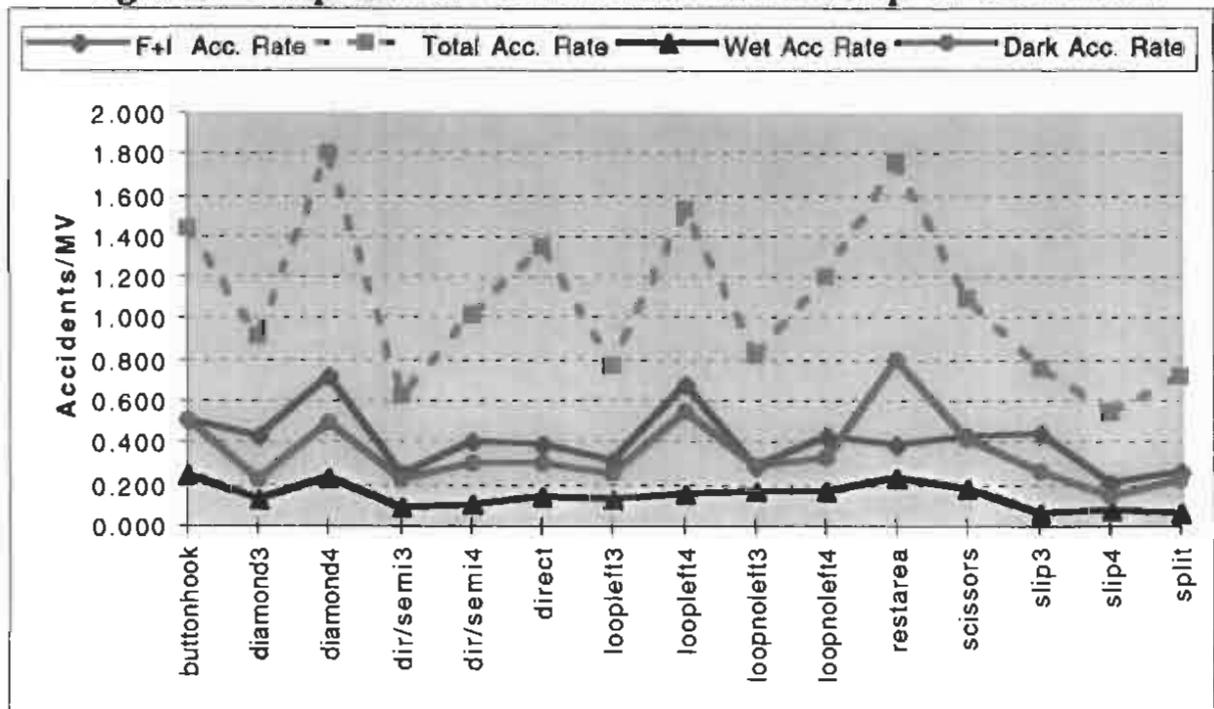
Because the ANCOVA analyses for urban areas (Appendix C) indicated district effect as being significant (meaning some accident rate differences in ramps attributed to Caltrans district and not just to ramp volumes), comparison tables for urban ramps were selected from ANCOVA analyses that accounted for the geographical effect (Caltrans Districts). Results for rural ramps were extracted from ANCOVA analyses with ramp volume as covariate, but without the district effect as a predictor in the model.

Figure 16 Comparison of Accident Rates for Off-Ramps in Rural Areas *



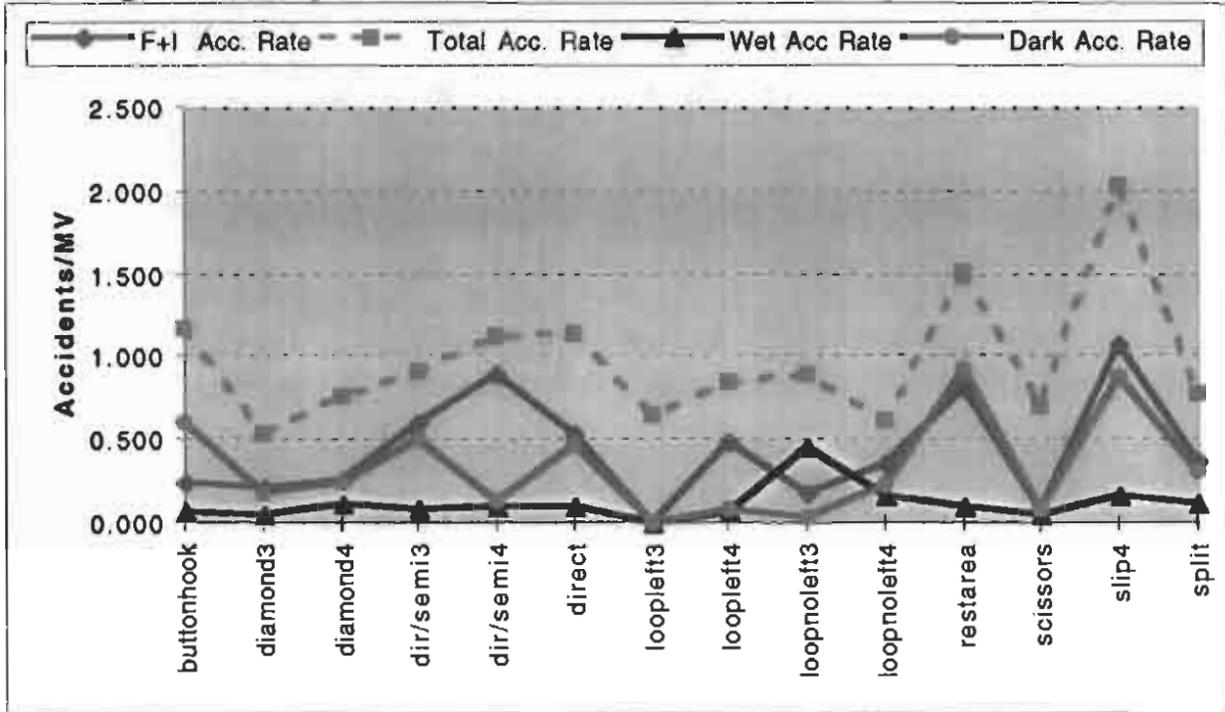
Notes: 1) For suffix 3 and 4 in ramp designation (i.e. loopleft3, ect.) see section 3.2.
 2) dir/semi = Direct or Semi-direct Connector; loopnoleft = Loop with no Left Turn; loopleft = Loop with Left Turn; buttonhook= Button Hook; twoway = Two-Way Ramp; restarea= Rest Areas, Vista Points and Truck Scales

Figure 17 Comparison of Accident Rates for Off-Ramps in Urban Areas



* (With Ramp Volume Adjusted)

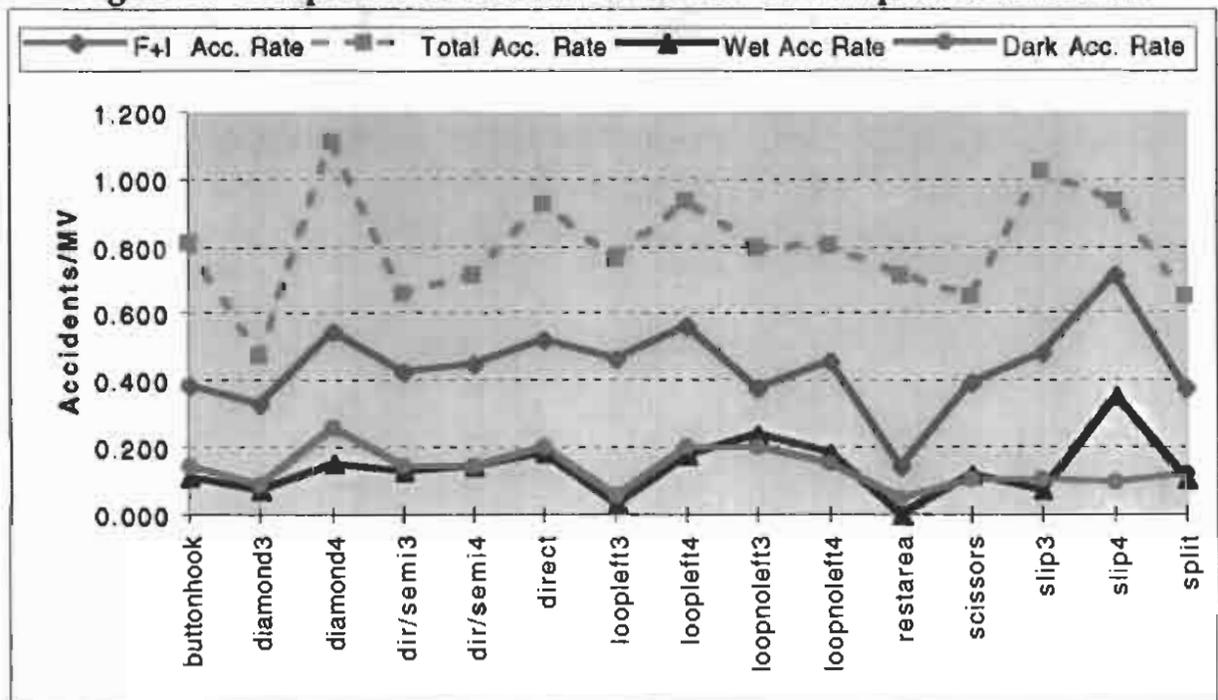
Figure 18 Comparison of Accident Rates for On-Ramps in Rural Areas



Notes: 1) For suffix 3 and 4 in ramp designation (i.e. loopleft3, ect.) see section 3.2.

2) dir/semi = Direct or Semi-direct Connector; loopnoleft = Loop with no Left Turn; loopleft = Loop with Left Turn; buttonhook= Button Hook; twoway = Two-Way Ramp; restarea= Rest Areas, Vista Points and Truck Scales

Figure 19 Comparison of Accident Rates for On-Ramps in Urban Areas



* (With Ramp Volume Adjusted)

Table 13 Comparison of Accident Rates for Off-Ramps in Rural Areas

	F+I Acc. Rate	Total Acc. Rate	Wet Acc Rate	Dark Acc. Rate
buttonhook	1.758	4.325	0.132	1.424
diamond3	0.348	1.057	0.028	0.452
diamond4	0.878	1.961	0.200	0.804
dir/semi3	1.111	2.005	0.027	0.845
dir/semi4	0.661	1.549	0.083	0.726
direct	0.590	1.328	0.055	0.564
loopleft3	0.952	5.833	0.278	1.843
loopleft4	0.995	3.881	0.062	0.462
loopnoleft3	0.385	1.531	0.099	0.783
loopnoleft4	4.568	4.745	0.059	0.129
restarea	0.301	2.472	0.055	1.198
scissors	0.750	1.627	0.081	0.816
slip4	0.000	0.000	0.000	0.000
split	0.984	2.106	0.090	0.980

Notes: 1) For suffix 3 and 4 in ramp designation (i.e. loopleft3, ect.) see section 3.2.

2) dir/semi = Direct or Semi-direct Connector; loopnoleft = Loop with no Left Turn; loopleft = Loop with Left Turn; buttonhook= Button Hook; twoway = Two-Way Ramp; restarea= Rest Areas, Vista Points and Truck Scales

Table 14 Comparison of Accident Rates for Off-Ramps in Urban Areas

	F+I Acc. Rate	Total Acc. Rate	Wet Acc Rate	Dark Acc. Rate
buttonhook	0.503	1.435	0.243	0.506
diamond3	0.429	0.921	0.126	0.221
diamond4	0.724	1.808	0.239	0.501
dir/semi3	0.246	0.633	0.085	0.229
dir/semi4	0.399	1.022	0.102	0.304
direct	0.393	1.352	0.148	0.296
loopleft3	0.307	0.765	0.126	0.247
loopleft4	0.684	1.531	0.162	0.546
loopnoleft3	0.293	0.821	0.171	0.288
loopnoleft4	0.436	1.197	0.164	0.323
restarea	0.398	1.759	0.238	0.793
scissors	0.431	1.096	0.184	0.424
slip3	0.441	0.764	0.066	0.266
slip4	0.211	0.554	0.085	0.145
split	0.257	0.717	0.059	0.221

Table 15 Comparison of Accident Rates for On-Ramps in Rural Areas

	F+I Acc. Rate	Total Acc. Rate	Wet Acc Rate	Dark Acc. Rate
buttonhook	0.243	1.172	0.061	0.598
diamond3	0.218	0.532	0.042	0.176
diamond4	0.256	0.761	0.110	0.243
dir/semi3	0.600	0.914	0.084	0.498
dir/semi4	0.889	1.127	0.090	0.129
direct	0.528	1.137	0.091	0.473
loopleft3	0.002	0.657	0.000	0.002
loopleft4	0.480	0.840	0.062	0.076
loopnoleft3	0.175	0.886	0.455	0.029
loopnoleft4	0.356	0.624	0.167	0.221
restarea	0.814	1.518	0.091	0.925
scissors	0.080	0.703	0.042	0.081
slip4	1.064	2.023	0.160	0.879
split	0.379	0.777	0.114	0.313

Notes : 1) For suffix 3 and 4 in ramp designation (i.e. loopleft3, ect.) see section 3.2.

2) dir/semi = Direct or Semi-direct Connector; loopnoleft = Loop with no Left Turn; loopleft = Loop with Left Turn; buttonhook= Button Hook; twoway = Two-Way Ramp; restarea= Rest Areas, Vista Points and Truck Scales

Table 16 Comparison of Accident Rates for On-Ramps in Urban Areas

	F+I Acc. Rate	Total Acc. Rate	Wet Acc Rate	Dark Acc. Rate
buttonhook	0.389	0.817	0.109	0.144
diamond3	0.327	0.473	0.075	0.088
diamond4	0.546	1.108	0.153	0.261
dir/semi3	0.426	0.663	0.129	0.147
dir/semi4	0.451	0.713	0.143	0.141
direct	0.520	0.928	0.186	0.201
loopleft3	0.468	0.762	0.029	0.056
loopleft4	0.566	0.937	0.176	0.204
loopnoleft3	0.381	0.798	0.242	0.198
loopnoleft4	0.457	0.807	0.185	0.151
restarea	0.145	0.714	0.000	0.046
scissors	0.396	0.653	0.119	0.105
slip3	0.486	1.023	0.075	0.106
slip4	0.715	0.938	0.352	0.098
split	0.382	0.649	0.101	0.122

4.7 Ramps with Relatively High Accident Rates

As stated earlier in this report, not all ramps experienced accidents. On the other hand, certain ramps did experience multiple accidents or had higher accident rates. This section provides some information on selected sample of ramps that have relatively high accident rates for future close monitoring purpose. The process by which these ramps were selected started by defining cutoffs for the fatal, fatal/injury and total accident rates (these cutoff points were specific to any ramp configuration under consideration), and then picked a handful of ramps that figured prominently on one or more of these lists. In essence, approximately the top 2.5% were analyzed here as a subset sample. A complete list of these ramps is provided in the Appendix D. Table 17 below summarizes information on high accident ramp. Appendix D provide the complete list of high accident ramps.

Table 17 Ramps with a relatively high accident rate

Ramp configuration	Ramp population (1)	High Rate Sample (2)	% [(2)/(1)] = (3)	% (2)/381	% (1)/13325
loopleft3	34	16	47.06 %	4.2%	0.3%
slip3	145	34	23.45 %	8.9%	1.1%
twowayseg4	32	6	18.75 %	1.6%	0.2%
Restarea	316	40	12.66 %	10.5%	2.4%
scissors	352	42	11.93 %	11.0%	2.6%
slip4	176	14	7.95 %	3.7%	1.3%
loopnoleft3	318	21	6.60 %	5.5%	2.4%
direct	470	24	5.11 %	6.3%	3.5%
diamond3	447	22	4.92 %	5.8%	3.4%
loopleft4	544	22	4.04 %	5.8%	4.1%
dir/semi3	738	23	3.12 %	6.0%	5.5%
split	870	21	2.41 %	5.5%	6.5%
loopnoleft4	881	21	2.38 %	5.5%	6.6%
buttonhook	1,179	27	2.29 %	7.1%	8.8%
dir/semi4	1,205	23	1.91 %	6.0%	9.0%
diamond4	5,618	25	0.44 %	6.6%	42.2%
Total Ramps	13,325	381	2.86 %	100%	100%

Notes: 1) For suffix 3 and 4 in ramp designation (i.e. loopleft3, ect.) see section 3.2.

2) dir/semi = Direct or Semi-direct Connector; loopnoleft = Loop with no Left Turn; loopleft = Loop with Left Turn; buttonhook= Button Hook; twoway = Two-Way Ramp; restarea= Rest Areas, Vista Points and Truck Scales

Notice that the scissors, rest area, and slip3 ramp configurations are the most common ramp among the ramp sample with a high accident rate. They comprise 11, 10, and 9 percent of the sampled 382 ramps respectively. Although, the percentage of the loop ramps with a left turn is relatively low in the sample, it comprises an overwhelming portion of this specific ramp configuration (47%). The other three types of ramps with over 10% of their population present in the sample are: slip3-ramp (23.45%), the two-waySeg4 ramp (18.75%), and the rest area/truck scale/vista point type of ramp (12.66%).

The rural/urban split for high accident sample is approximately 30/70, which is slightly different than the rural/urban split ratio (20/80) for all ramps population. From table 18, almost 80% of all ramps are found in an urban location, while only 70% of the ramps in the sample are high accident rates ramps.

Table 18 High Accident Rate Ramp Sample Distribution by Rural/Urban

Ramp Group	All Ramps	Percent of All Ramps	Sampled High Acc. Rate	Percent of Total Sample
Rural	2,739	20.6%	113	29.7%
Urban	10,586	79.4%	268	70.3%
Total	13,325	100.0%	381	100.0%

This may also indicates that rural ramps have a higher share of the ramps with high accident rates than urban ramps. The distribution between the ramps with relatively high rates grouped by on/off ramps is not much different than the distribution found for all the ramps. As seen in the table 19, off-ramps comprise the majority of the high accident rate ramp sample, at almost 51%. At first glance this may seem to contradict the previous findings of this report that off-ramps have higher accident rates. However, a closer look at the variability of accident occurrence between different ramp configurations and within each configuration

clarifies this point. Although, off-ramps have higher accident rates than on-ramps in their aggregate form the vast variability among and within each configuration lead to an equal selected sample size of on-ramps and off-ramps which have higher accident rates among the entire 13, 325 ramps in California.

Table 19 Higher Accident Rate Ramp Sample--Distribution by Off/On

Ramp Group	Total population	% of population	Higher Acc. Rate Sample	% of Sample
Off	6,672	50.1%	193	50.9%
On	6,621	49.7%	181	47.5%
Two-Way Ramp	32	0.2%	6	1.6%
Total	13,325	100.0%	381	100.0%

Another important observation from table 19 is that while the top five ramps comprise a very small portion of the ramp population, they make relatively a much higher percentage of the high accident ramp sample and especially a much higher percentage of their corresponding ramp configuration (e.g. two-way ramps make up only 0.2% of the entire ramp population, 1.6% of high accident rate sample, and over 18% of two-way ramps). Furthermore, two-way ramp segments stand out as being problematic, as seen earlier in the high accident rate ramp.

Findings

Approximately 15% of accidents on California state highways occurred on ramps. The rural ramp accidents were only 2.82% of accidents on rural highways, but the urban ramps accounted for 18.41% of the accidents on urban highways. The majority of ramps experienced no multiple-vehicle accidents or only one multiple-vehicle accident in the study period.

Accident rates of on-ramps were consistently lower than off-ramps accident rates as summarized in tables 13 through 16. Most fatal and injury type accidents occur on off-ramps, but on-ramps tended to be linked with the less severe PDO accidents. Not only do most ramp accidents occur on the off-ramps, but so do the more serious accidents. Fatal accidents constituted between 0.4 to 1.4 percent of ramp accidents; injury accidents between 34 to 38.8 percent, and property damage accidents between 60.8 to 65 percent as detailed in Table 8.

Descriptive statistics for every ramp configuration is derived and presented in Appendix B of this report. A series of analysis of variance models were run to determine the systematic differences in accident rates between ramps of different designs. These **statistical models** included the measured ramp traffic volume as a covariate and ramp geographical location as a predictor variable. The statistical models were followed by pairwise comparison where differences in mean accident rates between each pair of ramp configurations were estimated (see Appendix C). The comparisons are done for four sets of ramp groups stratified by ramp type (on/off), and ramp area (urban/rural) as presented in figure 16 through 19 and corresponding tables 13 through 16.

A list of ramps on California state highways with relatively higher than average accidents are summarized in table 17. Scissor ramps, rest area ramps, and slip ramps configurations are the most common among the ramp sample with high accident rates. They comprised 11, 10, and 9 percent of the high accident ramp sample respectively. Although, the percentage of the loop ramps with a

left turn is relatively low in the sample of ramp configurations, it comprised an overwhelming portion of its own ramp configuration (47%). The other three types of ramps with over 10% of their population present in the sample were: slip-ramps (23.45%), the two-way ramps (18.75%), and the rest area/truck scale/vista point ramps (12.66%).

The independent variables most often found by studies to be statistically significant were: ramp AADT, mainline freeway AADT, area type (rural/urban), ramp type (off/on), ramp configuration, length of SCL, ramp length. Ramp geometry in studies that developed accident prediction models found to be a much weaker variable than main line traffic volume and ramp traffic volume. The studies did not suggest that geometric variables are unimportant, rather geometric factors are generally improved to a point where further variance in geometric features has little influence on accidents. The general consensus of many studies is that traffic volume is the strongest predictor of accidents on ramps. Potential variables affecting accident occurrence on acceleration/deceleration lanes according to a study were: ramp AADT; mainline freeway AADT; average lane width; right shoulder width; acceleration/deceleration lane length; ramp area type (urban/rural), and; ramp configuration. The key variables were ramp AADT, main line AADT, and SCL length. Higher traffic volumes were associated with higher accidents. The effect of acceleration lane length on accident rate was significant when merging traffic percentage exceeded 6 percent of the main line traffic volume. As the percentage of merging traffic increases beyond this volume, the additional length of acceleration lane provides a significant reduction in accidents. The effect was not as great for deceleration lanes compared to acceleration lane. Truck accident studies are even more adamant about speed change lane, claiming that the current AASHTO requirements for deceleration lane lengths are not sufficient for trucks.

As a whole, research findings indicated that it is better to design ramps with flat horizontal curves (except in rural areas) avoiding the maximum degree of curvature, speed, and superelevation. Sharp curves at the end of the ramps and sudden changes from a straight alignment to sharp curves should be avoided. Ramp curves tend to be more critical than highway curves. Studies indicated that except for loop ramps in rural areas, all right-hand side and outer-connection ramps showed an increase in accident rates with increasing curvature. Outer-connection ramps in urban areas showed an increase in accident rates with increasing average daily traffic (ADT). Straight outer-connections have lower accident rates than curved connections in urban and rural areas for all ADTs, except 0 to 499 in urban areas. Rural loops with low curvature have higher accident rates than rural loops with high curvature, while the reverse is true for urban loops. In designing horizontal curves to accommodate trucks, it is important to check for both rollover and skidding potential to determine which controls the design. Abrupt changes in compound curves is associated with truck accidents. Many tank truck overturns were produced on ramps where the sharpness of the curve increased at the end of the ramp.

A good portion of the accidents associated with ramps occurs at the entrance or exit of ramps. Entrance terminal accidents estimated by a previous study to be as high as 52% for the on-ramps. About 44 percent of the off-ramp accidents occurred in the diverging area. While additional lanes improve the safety at entrance terminals, studies indicated no safety benefits as a result of auxiliary lanes at off-ramps compared to direct off-ramp designs. Studies found off-ramps with speed-change lanes have a lower accident rate than those ramps without speed-change lanes. Deceleration lanes are generally safer than acceleration lanes, regardless of the length of the speed change lanes and the percentage of diverging and merging traffic.

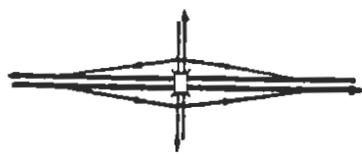
The most common accident in the entrance terminal is the rear-end collision. These accidents can be reduced by changing the merging maneuver at the entrance terminal to a less complex lane-changing maneuver through the use of a parallel acceleration lane. Other studies found that rear-end accidents on urban off-ramps of four configurations (diamond, parclo loop, free-flow loop, and outer connection ramps) were generally related to the operation of the cross-road ramp terminal, rather than to the geometric design of the ramp itself.

Steeper grade (positive or negative) can be associated with increased accident rates. As the vertical grade of a highway increases, the accident-involvement rate also increases (depending on the length of the grade). Substantial downgrades leading to tight ramp curves contribute to traffic accidents. More conservative grade allowances of 3 or 4% for ramp down grades compared to the 8% found in the AASHTO "green book" has been suggested.

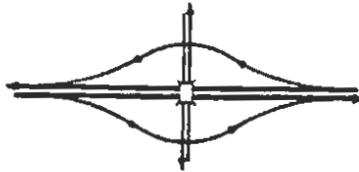
Truck loss of control on ramps is predominantly due to rollover and jackknifing. Jackknife accidents occur mainly at sites with low pavement friction during wet condition. Truck rollover accidents occur on ramps where truck speed is higher than ramp design speed. Design of ramps must also provide for adequate width to accommodate offtracking during low-speed and high-speed. Lowered friction levels on high-speed ramps can contribute to truck accidents. Therefore, it recommends the resurfacing of ramps with high-friction overlays. Poor transitions to superelevation contribute to truck accidents, such as rollovers and jackknifing. Regardless of the urban/rural location of the highway, trucks have more accidents on the off-ramps than on the on-ramps.

Finally, a separate analysis of half diamond ramps with discussion of analysis results is presented in Appendix F.

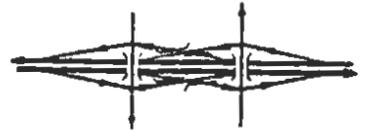
Appendix A



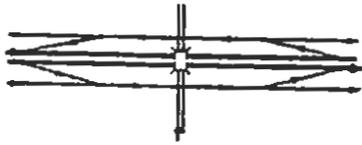
Compact diamond type



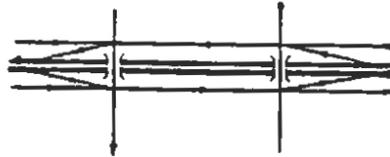
Spread diamond type



Split diamond braids



Parallel Street Systems



Parallel Street Systems



Parallel Street Systems



Slip Off-Ramp



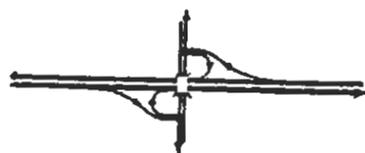
Buttonhook Off-Ramp



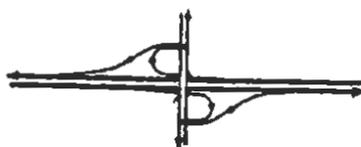
Scissors Ramp

Figure 1 Typical Local Street Interchanges

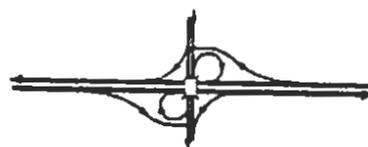
Appendix A



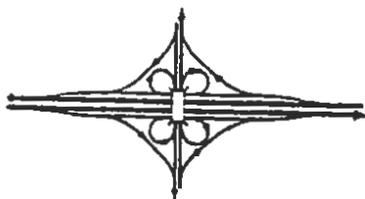
Parclo Interchanges



Parclo Interchanges



Parclo Interchanges



Cloverleaf Interchanges



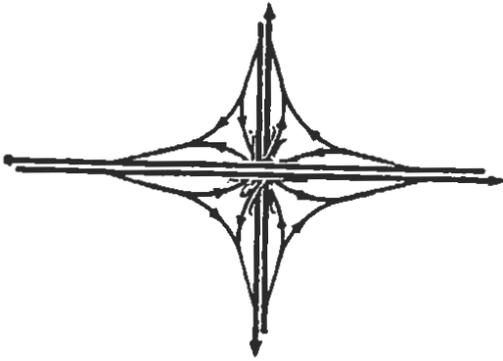
Trumpet



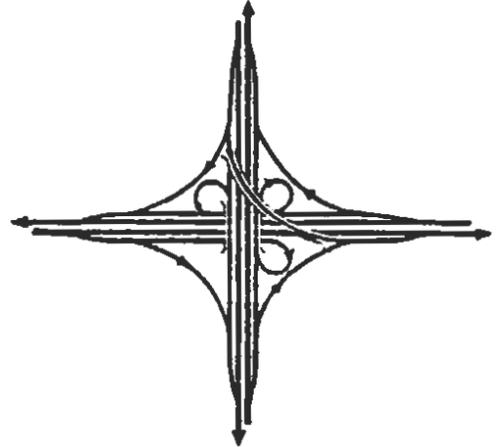
Trumpet

Figure 2 Typical Local Street Interchanges

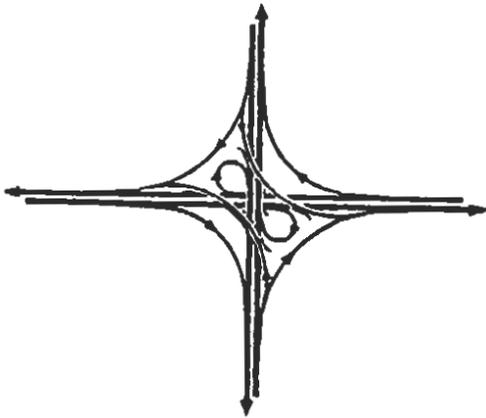
Appendix A



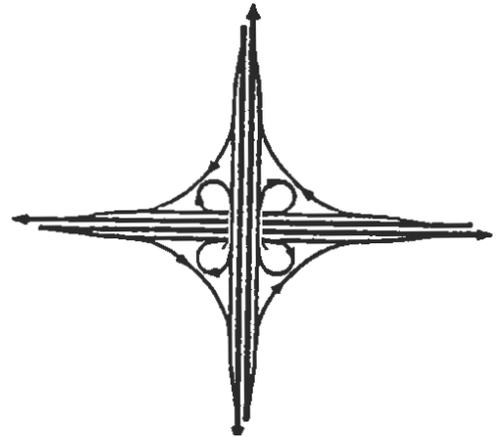
Direct connections



Three quadrant cloverleaf



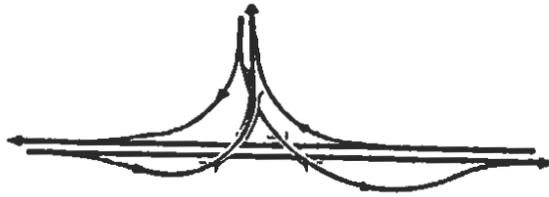
Two-loop, two-direct connection



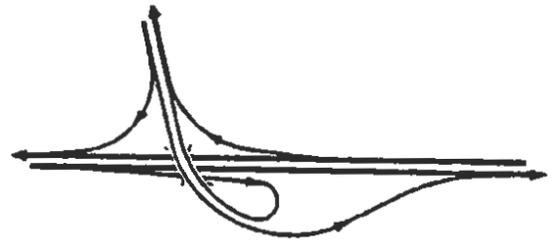
Cloverleaf with collector-distributor road

Figure 3 Typical Freeway-to-Freeway Interchange

Appendix A



Freeway Terminal Junction



Freeway Terminal Junction



Freeway Terminal Junction



Freeway Terminal Junction

Figure 4 Typical Freeway-to-Freeway Interchange

Appendix B

Descriptive Statistics on Individual Ramp Configurations

Note: 60 tables provided in this Appendix provide detail information on 15 ramp configurations. (Each configuration occurs in urban and rural setting and as on-ramp or off-ramp, thus $15 \times 2 \times 2 = 60$).

Each table precedes with a line of information specifying the ramp configuration and information on ramp type (on/off) and area location (urban/ rural setting). The following legends apply to each table.

*** Legend**

N	= Number of ramps for this configuration
TACC	= Total Accidents
FACC	= Fatal Accidents
IACC	= Injury Accidents
FIACC	= Fatal + Injury Accidents
MVACC	= Multi-vehicle Accidents
WACC	= Wet Accidents
DACC	= Dark Accidents
XVOL	= Ramp Cross Street Traffic Volume
RVOL	= Ramp Traffic Volume
FRATE	= Fatal Accident Rate
FIRATE	= Fatal + Injury Accident Rate
TRATE	= Total Accident Rate
WRATE	= Wet Accident Rate
DRATE	= Dark Accident Rate

Ramp Type= Off Urban/Rural =Rural DESCR=buttonhook

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	111	1.3513514	1.7666025	0	8.0000000
FACC	111	0.0090090	0.0949158	0	1.0000000
IACC	111	0.4774775	0.9327072	0	5.0000000
FIACC	111	0.4864865	0.9328828	0	5.0000000
MVACC	111	0.5585586	1.3258595	0	8.0000000
WACC	111	0.0630631	0.2441787	0	1.0000000
DACC	111	0.3963964	0.6505841	0	2.0000000
XVOL	111	0.7531532	0.8146187	0	3.7000000
RVOL	111	0.8185586	0.8891281	0.0200000	4.0500000
FRATE	111	0.0433063	0.4562602	0	4.8070000
FIRATE	111	1.6695495	8.1557984	0	82.1900000
TRATE	111	4.1284685	11.3893260	0	82.1900000
WRATE	111	0.1269562	0.7101832	0	6.5200000
DRATE	111	1.3286379	5.5941815	0	54.7933333

Ramp Type= Off Urban/Rural =Rural DESCR=diamond3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	92	1.0108696	1.8126210	0	9.0000000
FACC	92	0	0	0	0
IACC	92	0.3260870	0.7428590	0	4.0000000
FIACC	92	0.3260870	0.7428590	0	4.0000000
MVACC	92	0.7717391	1.5342483	0	7.0000000
WACC	92	0.0652174	0.2482620	0	1.0000000
DACC	92	0.2608696	0.6266879	0	3.0000000
XVOL	92	1.3391304	1.5638749	0	7.5000000
RVOL	92	1.4567391	1.7100402	0.0200000	8.2100000
FRATE	92	0	0	0	0
FIRATE	92	0.1832609	0.5639307	0	4.3500000
TRATE	92	0.6898913	1.4253101	0	9.4500000
DIST	92	5.8369565	3.1000786	1.0000000	11.0000000
WRATE	92	0.0184530	0.0757196	0	0.4950000
DRATE	92	0.2745892	1.2458816	0	9.4500000

Ramp Type= Off Urban/Rural =Rural DESCR=diamond4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	841	1.0927467	2.0790239	0	17.0000000
FACC	841	0.0190250	0.1366940	0	1.0000000
IACC	841	0.3828775	0.8470632	0	7.0000000
FIACC	841	0.4019025	0.8702469	0	7.0000000
MVACC	841	0.7205707	1.8045360	0	17.0000000
WACC	841	0.1379310	0.4472136	0	4.0000000
DACC	841	0.3305589	0.7512724	0	5.0000000
XVOL	841	0.8777646	1.2325516	0	9.3000000
RVOL	841	0.9472295	1.3464071	0	10.1800000
FRATE	841	0.0417015	0.4603627	0	8.2650000
FIRATE	841	0.7737931	3.1071513	0	45.6600000
TRATE	841	1.7295006	4.3968616	0	45.6600000
WRATE	841	0.1937981	1.0447615	0	15.6600000
DRATE	841	0.6928683	2.7112742	0	36.5300000

Ramp Type= Off Urban/Rural =Rural DESCR=dir/semi3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	40	0.8000000	1.1591332	0	5.0000000
FACC	40	0	0	0	0
IACC	40	0.4500000	0.8458041	0	4.0000000
FIACC	40	0.4500000	0.8458041	0	4.0000000
MVACC	40	0.2250000	0.6196566	0	3.0000000
WACC	40	0.0500000	0.2207214	0	1.0000000
DACC	40	0.3500000	0.5334936	0	2.0000000
XVOL	40	3.3575000	5.3689940	0.1000000	27.0000000
RVOL	40	3.5972500	5.9066985	0.0900000	29.5700000
FRATE	40	0	0	0	0
FIRATE	40	0.6912500	2.5939448	0	15.2200000
TRATE	40	1.0672500	2.9171298	0	15.2200000
WRATE	40	0.0030833	0.0137414	0	0.0700000
DRATE	40	0.3922917	1.5071332	0	9.4500000

Appendix B Descriptive Statistics on Individual Ramp Configurations

Ramp Type= Off Urban/Rural =Rural DESCR=dir/semi4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	49	1.2448980	1.7740093	0	7.0000000
FACC	49	0	0	0	0
IACC	49	0.4693878	0.7932539	0	3.0000000
FIACC	49	0.4693878	0.7932539	0	3.0000000
MVACC	49	0.4693878	0.8190961	0	3.0000000
WACC	49	0.1224490	0.4844521	0	3.0000000
DACC	49	0.4489796	0.6788846	0	2.0000000
XVOL	49	1.9224490	2.6810692	0.1600000	13.3000000
RVOL	49	2.1032653	2.9417210	0.0700000	14.6000000
FRATE	49	0	0	0	0
FIRATE	49	0.4195918	1.4801829	0	10.1500000
TRATE	49	1.0097959	2.0473382	0	10.1500000
WRATE	49	0.0688309	0.3027513	0	1.9600000
DRATE	49	0.4662119	1.4515969	0	9.7800000

Ramp Type= Off Urban/Rural =Rural DESCR=direct

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	19	2.2105263	2.7804665	0	9.0000000
FACC	19	0.0526316	0.2294157	0	1.0000000
IACC	19	0.9473684	1.2681432	0	4.0000000
FIACC	19	1.0000000	1.3743685	0	4.0000000
MVACC	19	0.8421053	1.1672931	0	4.0000000
WACC	19	0.1578947	0.3746343	0	1.0000000
DACC	19	0.8421053	1.2588865	0	4.0000000
XVOL	19	2.8578947	1.8771479	0.1000000	6.1000000
RVOL	19	3.0500000	1.9432533	0.0700000	6.6800000
FRATE	19	0.0085789	0.0373948	0	0.1630000
FIRATE	19	0.2352632	0.2969917	0	0.9900000
TRATE	19	0.5363158	0.5852370	0	2.0300000
WRATE	19	0.0349781	0.0846815	0	0.2537500
DRATE	19	0.1820175	0.2408641	0	0.6533333

Ramp Type= Off Urban/Rural =Rural DESCR=loopleft3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	4	4.0000000	3.5590261	1.0000000	8.0000000
FACC	4	0.2500000	0.5000000	0	1.0000000
IACC	4	0.5000000	0.5773503	0	1.0000000
FIACC	4	0.7500000	0.9574271	0	2.0000000
MVACC	4	1.2500000	1.2583057	0	3.0000000
WACC	4	0.2500000	0.5000000	0	1.0000000
DACC	4	1.5000000	1.9148542	0	4.0000000
XVOL	4	0.5500000	0.2645751	0.2000000	0.8000000
RVOL	4	0.6150000	0.2859487	0.2400000	0.9100000
FRATE	4	0.2740000	0.5480000	0	1.0960000
FIRATE	4	0.8875000	1.0793632	0	2.1900000
TRATE	4	5.6900000	3.3413470	1.7600000	8.7700000
WRATE	4	0.2740625	0.5481250	0	1.0962500
DRATE	4	1.7745833	2.1597700	0	4.3850000

Ramp Type= Off Urban/Rural =Rural DESCR=loopleft4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	25	2.1600000	4.4128600	0	21.0000000
FACC	25	0.0400000	0.2000000	0	1.0000000
IACC	25	0.8400000	2.0550750	0	10.0000000
FIACC	25	0.8800000	2.0477630	0	10.0000000
MVACC	25	1.5200000	4.3312816	0	21.0000000
WACC	25	0.1600000	0.5537749	0	2.0000000
DACC	25	0.2400000	0.4358899	0	1.0000000
XVOL	25	1.2320000	1.8254497	0	8.5000000
RVOL	25	1.3372000	2.0013215	0.0300000	9.2900000
FRATE	25	0.0332000	0.1660000	0	0.8300000
FIRATE	25	0.8444000	1.5678491	0	5.5900000
TRATE	25	3.5464000	7.9889647	0	39.1300000
WRATE	25	0.0533511	0.1883771	0	0.7960000
DRATE	25	0.3001137	0.8737495	0	3.5133333

Ramp Type= Off Urban/Rural =Rural DESCR=loopnoleft3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	14	1.2142857	2.2592910	0	8.0000000
FACC	14	0	0	0	0
IACC	14	0.4285714	1.0894096	0	4.0000000
FIACC	14	0.4285714	1.0894096	0	4.0000000
MVACC	14	0.1428571	0.3611365	0	1.0000000
WACC	14	0.2142857	0.5789342	0	2.0000000
DACC	14	0.6428571	1.4468609	0	4.0000000
XVOL	14	1.1928571	1.0380931	0	3.6000000
RVOL	14	1.3014286	1.1350151	0.0400000	3.9400000
FRATE	14	0	0	0	0
FIRATE	14	0.2385714	0.6189055	0	2.2800000
TRATE	14	1.2050000	2.5930907	0	9.4500000
WRATE	14	0.0905357	0.2492068	0	0.8775000
DRATE	14	0.6257143	1.3901520	0	4.7250000

Ramp Type= Off Urban/Rural =Rural DESCR=loopnoleft4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	11	0.6363636	1.2060454	0	3.0000000
FACC	11	0.0909091	0.3015113	0	1.0000000
IACC	11	0.4545455	0.9341987	0	3.0000000
FIACC	11	0.5454545	1.0357255	0	3.0000000
MVACC	11	0.0909091	0.3015113	0	1.0000000
WACC	11	0.0909091	0.3015113	0	1.0000000
DACC	11	0.0909091	0.3015113	0	1.0000000
XVOL	11	0.6000000	0.7523297	0	2.4000000
RVOL	11	0.6572727	0.8327916	0.0200000	2.6600000
FRATE	11	0.0915455	0.3036219	0	1.0070000
FIRATE	11	4.4990909	13.6730688	0	45.6600000
TRATE	11	4.5909091	13.6580653	0	45.6600000
WRATE	11	0.0551515	0.1829169	0	0.6066667
DRATE	11	0.0551515	0.1829169	0	0.6066667

Ramp Type= Off Urban/Rural =Rural DESCR=restarea

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	119	1.4453782	2.2349211	0	11.0000000
FACC	119	0.0084034	0.0916698	0	1.0000000
IACC	119	0.1512605	0.4041869	0	2.0000000
FIACC	119	0.1596639	0.4113474	0	2.0000000
MVACC	119	0.9495798	1.7066606	0	9.0000000
WACC	119	0.0924370	0.2908665	0	1.0000000
DACC	119	0.6134454	1.1055273	0	7.0000000
XVOL	119	1.3193277	1.4120713	0	6.3000000
RVOL	119	1.4410924	1.5374653	0	6.9000000
FRATE	119	0.0020168	0.0220008	0	0.2400000
FIRATE	119	0.1384874	0.5055315	0	3.1100000
TRATE	119	2.1096639	7.7883665	0	78.2700000
WRATE	119	0.0453494	0.1522043	0	0.7000000
DRATE	119	1.0227620	4.1060696	0	39.1350000

Ramp Type= Off Urban/Rural =Rural DESCR=scissors

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	37	1.7027027	2.2343886	0	8.0000000
FACC	37	0.0270270	0.1643990	0	1.0000000
IACC	37	0.7837838	1.0576477	0	3.0000000
FIACC	37	0.8108108	1.0498105	0	3.0000000
MVACC	37	1.0810811	1.8764384	0	7.0000000
WACC	37	0.1351351	0.3465835	0	1.0000000
DACC	37	0.6756757	1.1560001	0	6.0000000
XVOL	37	1.5864865	1.3254270	0.1000000	7.0000000
RVOL	37	1.7275676	1.4555324	0.0500000	7.6700000
FRATE	37	0.0667027	0.4057367	0	2.6600000
FIRATE	37	0.5532432	0.8686741	0	4.1200000
TRATE	37	1.1881081	2.4513317	0	14.4200000
WRATE	37	0.0692342	0.1871681	0	0.7900000
DRATE	37	0.6037741	2.0436827	0	12.3600000

Ramp Type= Off Urban/Rural =Rural DESCR=slip3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	0
FACC	0
IACC	0	0	.	.	.
FIACC	0
MVACC	0
WACC	0
DACC	0
XVOL	0
RVOL	0
FRATE	0
FIRATE	0
TRATE	0
WRATE	0
DRATE	0

Ramp Type= Off Urban/Rural =Rural DESCR=slip4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	1	0	.	0	0
FACC	1	0	.	0	0
IACC	1	0	.	0	0
FIACC	1	0	.	0	0
MVACC	1	0	.	0	0
WACC	1	0	.	0	0
DACC	1	0	.	0	0
XVOL	1	0.1000000	.	0.1000000	0.1000000
RVOL	1	0.0800000	.	0.0800000	0.0800000
FRATE	1	0	.	0	0
FIRATE	1	0	.	0	0
TRATE	1	0	.	0	0
WRATE	1	0	.	0	0
DRATE	1	0	.	0	0

Ramp Type= Off Urban/Rural =Rural DESCR=split

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	2	1.0000000	1.4142136	0	2.0000000
FACC	2	0	0	0	0
IACC	2	1.0000000	1.4142136	0	2.0000000
FIACC	2	1.0000000	1.4142136	0	2.0000000
MVACC	2	0.5000000	0.7071068	0	1.0000000
WACC	2	0.5000000	0.7071068	0	1.0000000
DACC	2	0	0	0	0
XVOL	2	7.0500000	6.7175144	2.3000000	11.8000000
RVOL	2	7.7000000	7.3821948	2.4800000	12.9200000
FRATE	2	0	0	0	0
FIRATE	2	0.0750000	0.1060660	0	0.1500000
TRATE	2	0.0750000	0.1060660	0	0.1500000
WRATE	2	0.0375000	0.0530330	0	0.0750000
DRATE	2	0	0	0	0

Ramp Type= Off Urban/Rural =Urban DESCR=buttonhook

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	499	5.4108216	5.6725899	0	42.0000000
FACC	499	0.0160321	0.1257248	0	1.0000000
IACC	499	1.8296593	2.3975058	0	19.0000000
FIACC	499	1.8456914	2.4111181	0	19.0000000
MVACC	499	3.2184369	4.3669971	0	30.0000000
WACC	499	0.8396794	1.6520490	0	24.0000000
DACC	499	1.8416834	2.3722344	0	25.0000000
XVOL	499	4.7458918	3.8627765	0	22.9000000
RVOL	499	5.0327455	4.2256676	0.0300000	25.0800000
FRATE	499	0.0114790	0.1288163	0	2.1230000
FIRATE	499	0.5677756	1.2226280	0	14.9900000
TRATE	499	1.6131864	2.6071293	0	33.7700000
WRATE	499	0.2694204	0.8534159	0	13.5080000
DRATE	499	0.5802522	1.3424963	0	18.2600000

Appendix B Descriptive Statistics on Individual Ramp Configurations

Ramp Type= Off Urban/Rural =Urban DESCR=diamond3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	160	6.6937500	6.3859091	0	37.0000000
FACC	160	0.0375000	0.1905800	0	1.0000000
IACC	160	2.6625000	2.9670889	0	19.0000000
FIACC	160	2.7000000	2.9922331	0	19.0000000
MVACC	160	5.1437500	5.5918257	0	35.0000000
WACC	160	0.9937500	1.7997095	0	12.0000000
DACC	160	1.7375000	2.1026713	0	12.0000000
XVOL	160	6.8331250	5.3123059	0.5000000	29.2000000
RVOL	160	7.3558750	5.8459524	0.5400000	32.0100000
FRATE	160	0.0111125	0.0856336	0	1.0150000
FIRATE	160	0.4668750	0.6671560	0	6.0900000
TRATE	160	1.1195625	1.3669823	0	9.8300000
WRATE	160	0.1518493	0.3454330	0	3.0100000
DRATE	160	0.2915496	0.3768905	0	1.9600000

Ramp Type= Off Urban/Rural =Urban DESCR=diamond4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	2193	9.3693570	10.8343706	0	99.0000000
FACC	2193	0.0259918	0.1619882	0	2.0000000
IACC	2193	3.9010488	4.6318640	0	46.0000000
FIACC	2193	3.9270406	4.6461627	0	46.0000000
MVACC	2193	7.9844961	10.1331346	0	98.0000000
WACC	2193	1.3210214	2.3668946	0	33.0000000
DACC	2193	2.5253078	3.0871500	0	22.0000000
XVOL	2193	6.1149111	4.6661472	0.1000000	45.0000000
RVOL	2193	6.5139535	5.0997772	0.0100000	49.2800000
FRATE	2193	0.0062535	0.0749332	0	2.9780000
FIRATE	2193	0.7665253	2.5679519	0	96.0800000
TRATE	2193	1.9674008	7.6767981	0	198.3900000
WRATE	2193	0.2645482	1.4683084	0	54.1050000
DRATE	2193	0.5596604	2.8390527	0	98.0400000

Ramp Type= Off Urban/Rural =Urban DESCR=dir/semi3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	363	5.4710744	8.7416015	0	117.0000000
FACC	363	0.0468320	0.2362451	0	2.0000000
IACC	363	2.0798898	3.0104253	0	26.0000000
FIACC	363	2.1267218	3.0266636	0	26.0000000
MVACC	363	2.7217631	4.7567626	0	32.0000000
WACC	363	1.0192837	1.9867413	0	14.0000000
DACC	363	2.3498623	5.0299016	0	82.0000000
XVOL	363	13.1752066	11.7355291	0.1000000	64.0000000
RVOL	363	13.9816253	12.9668736	0.0600000	70.0800000
FRATE	363	0.0035014	0.0211957	0	0.2370000
FIRATE	363	0.1884848	0.2661183	0	2.6000000
TRATE	363	0.4555372	0.6698474	0	9.1300000
WRATE	363	0.0832336	0.1470661	0	1.1175000
DRATE	363	0.1898803	0.3124045	0	3.6520000

Ramp Type= Off Urban/Rural =Urban DESCR=dir/semi4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	387	5.6692506	6.6946085	0	47.0000000
FACC	387	0.0310078	0.1878975	0	2.0000000
IACC	387	2.2144703	2.7500348	0	23.0000000
FIACC	387	2.2454780	2.7886147	0	23.0000000
MVACC	387	3.9431525	5.9385343	0	47.0000000
WACC	387	0.8087855	1.4122430	0	11.0000000
DACC	387	1.7545220	2.1878608	0	18.0000000
XVOL	387	6.1586563	4.8878401	0	34.0000000
RVOL	387	6.6673902	5.3139279	0.0500000	37.2300000
FRATE	387	0.0057364	0.0652740	0	1.2180000
FIRATE	387	0.4249354	0.6717873	0	7.6100000
TRATE	387	1.0619121	1.4592982	0	15.2200000
WRATE	387	0.1333667	0.2466853	0	1.6600000
DRATE	387	0.3272626	0.4918484	0	3.8066667

Ramp Type= Off Urban/Rural =Urban DESCR=direct

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	250	8.7640000	12.1543117	0	83.0000000
FACC	250	0.1160000	0.3889746	0	3.0000000
IACC	250	3.1440000	4.1762457	0	25.0000000
FIACC	250	3.2600000	4.3298256	0	27.0000000
MVACC	250	4.8600000	8.6405461	0	67.0000000
WACC	250	1.9360000	3.5403446	0	31.0000000
DACC	250	3.2000000	4.9194312	0	42.0000000
XVOL	250	15.1392000	13.2061224	0	80.0000000
RVOL	250	16.0500000	14.5995841	0	87.6000000
FRATE	250	0.0891360	0.0394637	0	0.3650000
FIRATE	250	0.3070800	0.8322630	0	10.0000000
TRATE	250	1.0975200	6.9351034	0	107.3300000
WRATE	250	0.1399853	0.3166131	0	3.7515789
DRATE	250	0.2323961	0.9410222	0	7.5031579

Ramp Type= Off Urban/Rural =Urban DESCR=loopleft3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	12	7.0000000	9.0453403	0	32.0000000
FACC	12	0	0	0	0
IACC	12	1.7500000	2.3011855	0	8.0000000
FIACC	12	1.7500000	2.3011855	0	8.0000000
MVACC	12	5.8333333	9.0134580	0	31.0000000
WACC	12	0.8333333	0.8348471	0	2.0000000
DACC	12	1.3333333	1.5669979	0	5.0000000
XVOL	12	5.5916667	5.9527623	0.5000000	22.0000000
RVOL	12	6.1141667	6.5253651	0.5300000	24.0900000
FRATE	12	0	0	0	0
FIRATE	12	0.3866667	0.5932244	0	1.9700000
TRATE	12	1.0100000	0.6693117	0	1.9700000
WRATE	12	0.1699033	0.2101437	0	0.5700000
DRATE	12	0.3134090	0.3906719	0	1.1400000

Ramp Type= Off Urban/Rural =Urban DESCR=loopleft4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	185	7.2810811	7.9205922	0	39.0000000
FACC	185	0.0162162	0.1266489	0	1.0000000
IACC	185	2.7297297	3.1142087	0	17.0000000
FIACC	185	2.7459459	3.1094696	0	17.0000000
MVACC	185	5.0270270	6.5958093	0	31.0000000
WACC	185	1.0162162	1.5758906	0	10.0000000
DACC	185	2.2810811	2.7695783	0	18.0000000
XVOL	185	5.3394595	3.9016036	0.1000000	22.0000000
RVOL	185	5.6358919	4.3454501	0.0800000	24.1300000
FRATE	185	0.0076054	0.0862270	0	1.1610000
FIRATE	185	0.7392432	2.1656808	0	27.4000000
TRATE	185	1.6637297	3.1605463	0	36.5300000
WRATE	185	0.1916954	0.3227278	0	2.3725000
DRATE	185	0.5983756	1.1866396	0	9.1325000

Ramp Type= Off Urban/Rural =Urban DESCR=loopnoleft3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	131	5.6717557	6.2590768	0	32.0000000
FACC	131	0.0229008	0.1501614	0	1.0000000
IACC	131	1.9083969	2.0132270	0	10.0000000
FIACC	131	1.9312977	2.0198661	0	10.0000000
MVACC	131	2.6412214	4.2154819	0	21.0000000
WACC	131	1.1374046	2.3195047	0	19.0000000
DACC	131	1.9618021	2.2647961	0	13.0000000
XVOL	131	6.9763359	5.3137388	0.1000000	28.0000000
RVOL	131	7.3060305	5.4927639	0.1000000	24.2000000
FRATE	131	0.0027939	0.0223051	0	0.2400000
FIRATE	131	0.3176336	0.4173042	0	2.2500000
TRATE	131	0.8915267	0.9091564	0	5.2600000
WRATE	131	0.2078579	0.5594739	0	4.9800000
DRATE	131	0.3152949	0.4076692	0	2.2500000

Appendix B Descriptive Statistics on Individual Ramp Configurations

Ramp Type= Off Urban/Rural =Urban DESCR=loopnoleft4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	255	7.5803922	9.5657900	0	82.0000000
FACC	255	0.0156863	0.1245030	0	1.0000000
IACC	255	2.7137255	3.5891909	0	30.0000000
FIACC	255	2.7294118	3.6090053	0	30.0000000
MVACC	255	5.3529412	8.8651834	0	78.0000000
WACC	255	1.2666667	2.1461068	0	17.0000000
DACC	255	2.0627451	2.4583107	0	17.0000000
XVOL	255	5.7266667	3.8102631	0.2000000	27.8000000
RVOL	255	6.1716471	4.1823531	0.1700000	30.4400000
FRATE	255	0.0041176	0.0403884	0	0.5710000
FIRATE	255	0.4759608	0.5579467	0	3.1100000
TRATE	255	1.2716863	1.2898174	0	8.9000000
WRATE	255	0.2029027	0.3089501	0	2.0300000
DRATE	255	0.3505607	0.4084097	0	2.4900000

Ramp Type= Off Urban/Rural =Urban DESCR=restarea

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	40	2.5750000	3.1040709	0	14.0000000
FACC	40	0.0250000	0.1581139	0	1.0000000
IACC	40	0.3750000	0.7741828	0	3.0000000
FIACC	40	0.4000000	0.7778999	0	3.0000000
MVACC	40	1.7000000	2.9369442	0	14.0000000
WACC	40	0.2000000	0.4050957	0	1.0000000
DACC	40	0.9250000	1.3085028	0	5.0000000
XVOL	40	2.2975000	2.5231734	0	9.6000000
RVOL	40	2.5100000	2.7606326	0	10.5100000
FRATE	40	0.0596000	0.3769435	0	2.3840000
FIRATE	40	0.5227500	1.5333188	0	9.0400000
TRATE	40	2.0845000	3.4797487	0	18.0800000
WRATE	40	0.2923453	1.4316195	0	9.0400000
DRATE	40	0.8984646	1.7703354	0	9.0400000

Ramp Type= Off Urban/Rural =Urban DESCR=scissors

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	135	4.9407407	4.9742065	0	23.0000000
FACC	135	0.0296296	0.1701948	0	1.0000000
IACC	135	1.7259259	2.0494980	0	11.0000000
FIACC	135	1.7555556	2.0605269	0	11.0000000
MVACC	135	2.8814815	3.7257712	0	18.0000000
WACC	135	0.8518519	1.5670036	0	9.0000000
DACC	135	1.7703704	2.0440695	0	12.0000000
XVOL	135	5.4192593	4.1649130	0.2000000	21.0000000
RVOL	135	5.9762222	4.9704275	0.1900000	23.0300000
FRATE	135	0.0069704	0.0479944	0	0.4810000
FIRATE	135	0.4678519	0.8034964	0	5.1700000
TRATE	135	1.1990370	1.4885929	0	7.4800000
WRATE	135	0.2167232	0.4840192	0	3.4250000
DRATE	135	0.4694709	0.8711206	0	6.2333333

Ramp Type= Off Urban/Rural =Urban DESCR=slip3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	79	2.6962025	4.0330507	0	17.0000000
FACC	79	0.0379747	0.1923564	0	1.0000000
IACC	79	0.8481013	1.3017602	0	7.0000000
FIACC	79	0.8860759	1.3396071	0	7.0000000
MVACC	79	1.9367089	3.4094205	0	16.0000000
WACC	79	0.5063291	1.2797578	0	9.0000000
DACC	79	0.9240506	1.5087110	0	7.0000000
XVOL	79	13.9556962	14.9395856	0.1000000	73.1000000
RVOL	79	14.8993671	16.5906629	0.0800000	80.0500000
FRATE	79	0.1152532	1.0172272	0	9.0420000
FIRATE	79	0.3558228	1.7000246	0	12.2900000
TRATE	79	0.5165823	1.7088059	0	12.2900000
WRATE	79	0.0436098	0.1188707	0	0.7000000
DRATE	79	0.2118783	1.0303544	0	9.0400000

Ramp Type= Off Urban/Rural =Urban DESCR=slip4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	96	3.3958333	3.9349424	0	19.0000000
FACC	96	0.0104167	0.1020621	0	1.0000000
IACC	96	1.2604167	1.4885160	0	7.0000000
FIACC	96	1.2708333	1.5112154	0	7.0000000
MVACC	96	2.5833333	3.2008771	0	16.0000000
WACC	96	0.5937500	1.4979152	0	11.0000000
DACC	96	1.2083333	1.7285020	0	9.0000000
XVOL	96	9.0281250	6.8731316	0.7000000	50.0000000
RVOL	96	9.7757292	7.6104671	0.7800000	54.7500000
FRATE	96	0.000427083	0.0041845	0	0.0410000
FIRATE	96	0.1864583	0.2947255	0	1.9100000
TRATE	96	0.4558333	0.6523152	0	4.4200000
WRATE	96	0.0759679	0.1799204	0	0.9100000
DRATE	96	0.1464949	0.2267652	0	1.1775000

Ramp Type= Off Urban/Rural =Urban DESCR=split

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	522	5.4521073	9.9112475	0	92.0000000
FACC	522	0.0363985	0.2069263	0	2.0000000
IACC	522	1.7605364	3.0159738	0	30.0000000
FIACC	522	1.7969349	3.0531049	0	30.0000000
MVACC	522	3.7432950	7.9378103	0	90.0000000
WACC	522	0.8812261	3.1036589	0	58.0000000
DACC	522	1.9923372	3.8273612	0	37.0000000
XVOL	522	19.6344828	15.2471576	0.1000000	99.5000000
RVOL	522	20.1913027	16.0025263	0.0900000	91.7600000
FRATE	522	0.0036954	0.0495117	0	1.0740000
FIRATE	522	0.1074138	0.3070790	0	3.7500000
TRATE	522	0.2833333	0.6468865	0	8.9200000
WRATE	522	0.0383917	0.1062506	0	1.3200000
DRATE	522	0.1021197	0.2384433	0	3.2200000

Ramp Type= On Urban/Rural =Rural DESCR=buttonhook

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	109	0.6055046	1.3334607	0	8.0000000
FACC	109	0	0	0	0
IACC	109	0.2110092	0.5785261	0	3.0000000
FIACC	109	0.2110092	0.5785261	0	3.0000000
MVACC	109	0.4587156	1.2287611	0	8.0000000
WACC	109	0.0550459	0.2291232	0	1.0000000
DACC	109	0.1192661	0.3528920	0	2.0000000
XVOL	109	0.9642202	1.1480679	0	5.5000000
RVOL	109	1.0495413	1.2534607	0.0200000	6.0600000
FRATE	109	0	0	0	0
FIRATE	109	0.2535780	1.0168265	0	7.9400000
TRATE	109	1.1911009	5.1214439	0	49.8200000
WRATE	109	0.0627248	0.4296977	0	4.3500000
DRATE	109	0.6067164	4.8017690	0	49.8200000

Ramp Type= On Urban/Rural =Rural DESCR=diamond3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	78	0.6025641	1.3420254	0	10.0000000
FACC	78	0	0	0	0
IACC	78	0.2307692	0.7546062	0	6.0000000
FIACC	78	0.2307692	0.7546062	0	6.0000000
MVACC	78	0.1666667	0.4082483	0	2.0000000
WACC	78	0.0512821	0.2743321	0	2.0000000
DACC	78	0.2435897	0.5390202	0	2.0000000
XVOL	78	1.3756410	1.2205157	0	4.6000000
RVOL	78	1.5082051	1.3355496	0.0200000	5.0700000
FRATE	78	0	0	0	0
FIRATE	78	0.1860256	0.6186377	0	4.1700000
TRATE	78	0.4715385	1.0527356	0	6.9600000
WRATE	78	0.0375897	0.2106962	0	1.3920000
DRATE	78	0.1502607	0.3966029	0	2.0800000

Appendix B Descriptive Statistics on Individual Ramp Configurations

Ramp Type= On Urban/Rural =Rural DESCR=diamond4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	759	0.4479578	1.0142093	0	9.0000000
FACC	759	0.0026350	0.0512988	0	1.0000000
IACC	759	0.1581028	0.4886950	0	5.0000000
FIACC	759	0.1607378	0.4932125	0	5.0000000
MVACC	759	0.2542819	0.7755529	0	7.0000000
WACC	759	0.0474308	0.2417299	0	3.0000000
DACC	759	0.1475626	0.4615464	0	4.0000000
XVOL	759	0.8096179	1.1399091	0	11.3000000
RVOL	759	0.8724769	1.2427067	0.0100000	12.3400000
FRATE	759	0.0068419	0.1649592	0	4.4910000
FIRATE	759	0.2826087	1.6227174	0	26.0900000
TRATE	759	0.8111199	3.5156941	0	68.4900000
WRATE	759	0.1144566	1.2941216	0	23.8300000
DRATE	759	0.2643806	1.5125195	0	23.8300000

Ramp Type= On Urban/Rural =Rural DESCR=dir/semi3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	41	0.9268293	1.4385799	0	6.0000000
FACC	41	0.0243902	0.1561738	0	1.0000000
IACC	41	0.4878049	1.2066887	0	6.0000000
FIACC	41	0.5121951	1.2066887	0	6.0000000
MVACC	41	0.2682927	0.5012180	0	2.0000000
WACC	41	0.1951220	0.4593208	0	2.0000000
DACC	41	0.3658537	0.8875837	0	5.0000000
XVOL	41	2.1878049	2.4860808	0	10.6000000
RVOL	41	2.3936585	2.7274665	0.0300000	11.6400000
FRATE	41	0.0032683	0.0209273	0	0.1340000
FIRATE	41	0.4875610	1.6754325	0	10.1500000
TRATE	41	0.6992683	1.7003785	0	10.1500000
WRATE	41	0.0674390	0.1655790	0	0.6100000
DRATE	41	0.4046341	1.6359082	0	10.1500000

Ramp Type= On Urban/Rural =Rural DESCR=dir/semi4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	65	0.6153846	1.1949413	0	7.0000000
FACC	65	0	0	0	0
IACC	65	0.2615385	0.7131997	0	4.0000000
FIACC	65	0.2615385	0.7131997	0	4.0000000
MVACC	65	0.2769231	0.9602684	0	7.0000000
WACC	65	0.1538462	0.4412613	0	2.0000000
DACC	65	0.2153846	0.5151549	0	2.0000000
XVOL	65	1.2969231	1.3753374	0	5.0000000
RVOL	65	1.3936923	1.4777248	0.0200000	5.4800000
FRATE	65	0	0	0	0
FIRATE	65	0.8681538	4.2318402	0	30.2100000
TRATE	65	1.0867692	4.2699580	0	30.2100000
WRATE	65	0.0872125	0.2900626	0	1.7171429
DRATE	65	0.1116484	0.2984670	0	1.7171429

Ramp Type= On Urban/Rural =Rural DESCR=direct

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	20	1.2000000	1.8524521	0	8.0000000
FACC	20	0	0	0	0
IACC	20	0.4000000	0.5026247	0	1.0000000
FIACC	20	0.4000000	0.5026247	0	1.0000000
MVACC	20	0.4500000	0.6048053	0	2.0000000
WACC	20	0.1500000	0.4893605	0	2.0000000
DACC	20	0.4000000	0.9403247	0	4.0000000
XVOL	20	4.9250000	6.3806348	0.2000000	25.5000000
RVOL	20	5.3905000	6.9885007	0.2300000	27.9200000
FRATE	20	0	0	0	0
FIRATE	20	0.1405000	0.3017401	0	1.3000000
TRATE	20	0.4000000	0.6008678	0	1.9400000
WRATE	20	0.0322500	0.1123959	0	0.4850000
DRATE	20	0.1535000	0.3589645	0	1.3000000

Appendix B Descriptive Statistics on Individual Ramp Configurations

Ramp Type= On Urban/Rural =Rural DESCR=loopleft3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	7	0.7142857	0.7559289	0	2.0000000
FACC	7	0	0	0	0
IACC	7	0	0	0	0
FIACC	7	0	0	0	0
MVACC	7	0.1428571	0.3779645	0	1.0000000
WACC	7	0	0	0	0
DACC	7	0	0	0	0
XVOL	7	1.1000000	0.9380832	0	2.6000000
RVOL	7	1.1828571	1.0056294	0.0500000	2.8100000
FRATE	7	0	0	0	0
FIRATE	7	0	0	0	0
TRATE	7	0.6528571	0.8438545	0	2.0000000
WRATE	7	0	0	0	0
DRATE	7	0	0	0	0

Ramp Type= On Urban/Rural =Rural DESCR=loopleft4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	72	0.5833333	0.9604870	0	4.0000000
FACC	72	0.0136889	0.1178511	0	1.0000000
IACC	72	0.2361111	0.4594257	0	2.0000000
FIACC	72	0.2500000	0.4964664	0	2.0000000
MVACC	72	0.4027778	0.7250007	0	3.0000000
WACC	72	0.0972222	0.2981392	0	1.0000000
DACC	72	0.1388889	0.3865906	0	2.0000000
XVOL	72	1.1013889	1.2942444	0	8.8000000
RVOL	72	1.2002778	1.4084333	0.0300000	9.6000000
FRATE	72	0.0044444	0.0377124	0	0.1200000
FIRATE	72	0.4768056	2.6967084	0	22.8300000
TRATE	72	0.8334722	2.8773159	0	22.8300000
WRATE	72	0.0619444	0.2298952	0	1.2850000
DRATE	72	0.0731944	0.2228091	0	1.0850000

Ramp Type= On Urban/Rural =Rural DESCR=loopnoleft3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	21	1.2380952	3.2543012	0	15.0000000
FACC	21	0	0	0	0
IACC	21	0.4285714	1.5352989	0	7.0000000
FIACC	21	0.4285714	1.5352989	0	7.0000000
MVACC	21	0.2380952	0.8890873	0	4.0000000
WACC	21	0.4285714	1.1212238	0	5.0000000
DACC	21	0.0952381	0.4364358	0	2.0000000
XVOL	21	1.3142857	2.7492337	0	10.0000000
RVOL	21	1.4338095	3.0242825	0.0300000	10.9900000
FRATE	21	0	0	0	0
FIRATE	21	0.1504762	0.4103959	0	1.6200000
TRATE	21	0.8385714	1.4136346	0	5.6000000
WRATE	21	0.4510317	1.2860530	0	5.6000000
DRATE	21	0.0086984	0.0398611	0	0.1826667

Ramp Type= On Urban/Rural =Rural DESCR=loopnoleft4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	34	0.5294118	1.4613256	0	7.0000000
FACC	34	0	0	0	0
IACC	34	0.1764706	0.4586270	0	2.0000000
FIACC	34	0.1764706	0.4586270	0	2.0000000
MVACC	34	0.1176471	0.3270350	0	1.0000000
WACC	34	0.2647059	1.1364171	0	6.0000000
DACC	34	0.0882353	0.2879022	0	1.0000000
XVOL	34	0.6970588	0.9366236	0	3.8000000
RVOL	34	0.7385294	1.0194477	0.0200000	4.1600000
FRATE	34	0	0	0	0
FIRATE	34	0.3950000	1.4724670	0	8.0600000
TRATE	34	0.6979412	1.8789533	0	8.0600000
WRATE	34	0.1729664	0.7524581	0	4.0380000
DRATE	34	0.2531513	1.3810035	0	8.0600000

Appendix B Descriptive Statistics on Individual Ramp Configurations

Ramp Type= On Urban/Rural =Rural DESCR=restarea

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	118	0.5762712	1.1723800	0	8.0000000
FACC	118	0	0	0	0
IACC	118	0.0593220	0.2372338	0	1.0000000
FIACC	118	0.0593220	0.2372338	0	1.0000000
MVACC	118	0.4067797	0.9717135	0	8.0000000
WACC	118	0.0254237	0.1580795	0	1.0000000
DACC	118	0.3050847	0.9291893	0	8.0000000
XVOL	118	1.3313559	1.3919610	0	6.3000000
RVOL	118	1.4538983	1.5154611	0.0100000	6.9000000
FRATE	118	0	0	0	0
FIRATE	118	0.7870339	7.6395954	0	82.9400000
TRATE	118	1.4667797	8.6516262	0	82.9400000
WRATE	118	0.0873051	0.8355936	0	9.0420000
DRATE	118	0.9024294	7.6404105	0	82.9400000

Ramp Type= On Urban/Rural =Rural DESCR=scissors

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	36	0.4166667	0.6491753	0	2.0000000
FACC	36	0	0	0	0
IACC	36	0.0833333	0.3683942	0	2.0000000
FIACC	36	0.0833333	0.3683942	0	2.0000000
MVACC	36	0.2500000	0.5000000	0	2.0000000
WACC	36	0.0555556	0.2323107	0	1.0000000
DACC	36	0.0555556	0.2323107	0	1.0000000
XVOL	36	1.1000000	0.9742396	0.1000000	3.8000000
RVOL	36	1.1969444	1.0651219	0.1100000	4.2000000
FRATE	36	0	0	0	0
FIRATE	36	0.0769444	0.3643297	0	2.1000000
TRATE	36	0.6972222	1.4240377	0	5.5300000
WRATE	36	0.0411111	0.1872016	0	1.0500000
DRATE	36	0.0780556	0.3372478	0	1.7600000

Ramp Type= On Urban/Rural =Rural DESCR=slip3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	0
FACC	0
IACC	0
FIACC	0
MVACC	0
WACC	0
DACC	0
XVOL	0
RVOL	0
FRATE	0
FIRATE	0
TRATE	0
WRATE	0
DRATE	0

Ramp Type= On Urban/Rural =Rural DESCR=slip4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	1	0	.	0	0
FACC	1	0	.	0	0
IACC	1	0	.	0	0
FIACC	1	0	.	0	0
MVACC	1	0	.	0	0
WACC	1	0	.	0	0
DACC	1	0	.	0	0
XVOL	1	11.7000000	.	11.7000000	11.7000000
RVOL	1	12.7800000	.	12.7800000	12.7800000
FRATE	1	0	.	0	0
FIRATE	1	0	.	0	0
TRATE	1	0	.	0	0
WRATE	1	0	.	0	0
DRATE	1	0	.	0	0

Appendix B Descriptive Statistics on Individual Ramp Configurations

Ramp Type= **On** Urban/Rural =**Rural** DESCR=**split**

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	3	0.3333333	0.5773503	0	1.0000000
FACC	3	0	0	0	0
IACC	3	0	0	0	0
FIACC	3	0	0	0	0
MVACC	3	0	0	0	0
WACC	3	0.3333333	0.5773503	0	1.0000000
DACC	3	0	0	0	0
XVOL	3	4.8666667	0.9291573	3.8000000	5.5000000
RVOL	3	5.3000000	0.9945351	4.1600000	5.9900000
FRATE	3	0	0	0	0
FIRATE	3	0	0	0	0
TRATE	3	0.0566667	0.0981495	0	0.1700000
WRATE	3	0.0566667	0.0981495	0	0.1700000
DRATE	3	0	0	0	0

Ramp Type= **On** Urban/Rural =**Urban** DESCR=**buttonhook**

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	460	2.8152174	4.4680647	0	51.0000000
FACC	460	0.0130435	0.1135843	0	1.0000000
IACC	460	1.0021739	1.7156195	0	11.0000000
FIACC	460	1.0152174	1.7269448	0	11.0000000
MVACC	460	2.2065217	4.0678827	0	49.0000000
WACC	460	0.4695652	1.0710835	0	14.0000000
DACC	460	0.7826087	1.2561490	0	8.0000000
XVOL	460	4.6158696	3.6481960	0	21.9000000
RVOL	460	4.8896304	3.9506893	0.0300000	23.9800000
FRATE	460	0.0045196	0.0583989	0	1.1710000
FIRATE	460	0.2289565	0.5204801	0	6.9900000
TRATE	460	0.7879348	2.8392068	0	54.7900000
WRATE	460	0.1044082	0.4671305	0	9.1316667
DRATE	460	0.2123784	0.7013309	0	9.1316667

Ramp Type= **On** Urban/Rural =**Urban** DESCR=**diamond3**

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	117	2.4529915	3.0300100	0	18.0000000
FACC	117	0.0256410	0.1587417	0	1.0000000
IACC	117	0.8547009	1.3407728	0	8.0000000
FIACC	117	0.8803419	1.3656020	0	8.0000000
MVACC	117	1.7008547	2.6822050	0	16.0000000
WACC	117	0.4258004	0.9213869	0	5.0000000
DACC	117	0.9059829	1.3832926	0	9.0000000
XVOL	117	5.6675214	4.3474493	0.1000000	26.5000000
RVOL	117	6.0610598	4.7823573	0.1500000	29.6200000
FRATE	117	0.0061282	0.0582421	0	0.5070000
FIRATE	117	0.1853846	0.3209188	0	1.7400000
TRATE	117	0.4611966	0.6186389	0	4.7800000
WRATE	117	0.0580635	0.1198827	0	0.6100000
DRATE	117	0.1630225	0.2665691	0	1.8300000

Ramp Type= **On** Urban/Rural =**Urban** DESCR=**diamond4**

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	1825	4.7452055	5.8692124	0	51.0000000
FACC	1825	0.0131507	0.1139511	0	1.0000000
IACC	1825	1.8410959	2.5887660	0	23.0000000
FIACC	1825	1.8542466	2.6010043	0	23.0000000
MVACC	1825	3.9041096	5.3041391	0	46.0000000
WACC	1825	0.7150685	1.6704389	0	18.0000000
DACC	1825	1.4043836	2.1720873	0	26.0000000
XVOL	1825	5.8280548	4.4483392	0	45.0000000
RVOL	1825	6.1903342	4.8358753	0.0100000	49.2800000
FRATE	1825	0.0032460	0.0490180	0	1.7790000
FIRATE	1825	0.3896110	1.3121398	0	30.4400000
TRATE	1825	1.0967123	5.9695134	0	234.4500000
WRATE	1825	0.1393905	0.6055333	0	18.0346156
DRATE	1825	0.3408819	2.3159457	0	90.1730769

Appendix B Descriptive Statistics on Individual Ramp Configurations

Ramp Type= On

Urban/Rural =Urban DESCR=dir/semi3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	294	4.5850340	6.4658130	0	51.0000000
FACC	294	0.0374150	0.2072775	0	2.0000000
IACC	294	1.6292517	2.3677212	0	23.0000000
FIACC	294	1.6666667	2.4448409	0	24.0000000
MVACC	294	2.2959184	4.1107763	0	43.0000000
WACC	294	1.0204082	2.6099557	0	28.0000000
DACC	294	1.8095238	2.5554154	0	14.0000000
XVOL	294	11.8612245	12.3476460	0	85.1000000
RVOL	294	12.4763265	13.5733202	0.0100000	93.1900000
FRATE	294	0.0034796	0.0320673	0	0.5070000
FIRATE	294	0.1915986	0.4393951	0	5.6200000
TRATE	294	0.4622789	0.7181631	0	5.7100000
WRATE	294	0.0983579	0.2673550	0	1.2000000
DRATE	294	0.1726593	0.2829240	0	2.8550000

Ramp Type= On

Urban/Rural =Urban DESCR=dir/semi4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	704	3.5042614	3.9919476	0	34.0000000
FACC	704	0.0156250	0.1350840	0	2.0000000
IACC	704	1.3281250	1.7354160	0	12.0000000
FIACC	704	1.3437500	1.7572498	0	12.0000000
MVACC	704	2.3764205	3.1800517	0	32.0000000
WACC	704	0.7116477	1.4264642	0	15.0000000
DACC	704	1.0440341	1.5611036	0	12.0000000
XVOL	704	5.6170455	4.0716512	0.1000000	32.3000000
RVOL	704	6.0266051	4.4793381	0.0600000	35.3700000
FRATE	704	0.0021974	0.0233021	0	0.4760000
FIRATE	704	0.2759233	0.4724064	0	4.8100000
TRATE	704	0.6806108	0.8568901	0	9.6100000
WRATE	704	0.1342215	0.3450026	0	4.8050000
DRATE	704	0.2063812	0.4460250	0	6.0900000

Ramp Type= On

Urban/Rural =Urban DESCR=direct

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	181	8.4254144	13.2296475	0	98.0000000
FACC	181	0.0994475	0.3512671	0	2.0000000
IACC	181	2.7624309	4.2653543	0	33.0000000
FIACC	181	2.8618785	4.3624324	0	35.0000000
MVACC	181	4.4198895	7.9036168	0	55.0000000
WACC	181	2.2928177	5.4444062	0	50.0000000
DACC	181	3.3535912	5.8534748	0	45.0000000
XVOL	181	14.3475138	11.0017048	0.7000000	52.0000000
RVOL	181	15.0644199	12.3210489	0.3100000	56.9400000
FRATE	181	0.0081326	0.0328428	0	0.2540000
FIRATE	181	0.2677901	0.9715478	0	12.7700000
TRATE	181	0.6595580	1.8737019	0	23.9400000
WRATE	181	0.1508528	0.3880652	0	3.3650000
DRATE	181	0.2113195	0.3354149	0	3.1920000

Ramp Type= On

Urban/Rural =Urban DESCR=loopleft3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	11	4.2727273	5.7635217	0	19.0000000
FACC	11	0	0	0	0
IACC	11	1.0000000	1.0000000	0	3.0000000
FIACC	11	1.0000000	1.0000000	0	3.0000000
MVACC	11	3.7272727	5.3120789	0	17.0000000
WACC	11	0.1818182	0.4045199	0	1.0000000
DACC	11	1.3636364	1.7477258	0	6.0000000
XVOL	11	5.7000000	5.4713801	0.4000000	18.2000000
RVOL	11	6.2418182	5.9968088	0.4700000	19.9300000
FRATE	11	0	0	0	0
FIRATE	11	0.3172727	0.6101490	0	2.1100000
TRATE	11	0.8627273	1.1964121	0	4.2100000
WRATE	11	0.0172727	0.0433799	0	0.1400000
DRATE	11	0.1805368	0.1524541	0	0.4933333

Appendix B Descriptive Statistics on Individual Ramp Configurations

Ramp Type= On

Urban/Rural =Urban DESCR=loopleft4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	262	4.6145038	5.8846071	0	49.0000000
FACC	262	0.0114504	0.1065957	0	1.0000000
IACC	262	1.7938931	2.6077326	0	27.0000000
FIACC	262	1.8053435	2.6122849	0	27.0000000
MVACC	262	3.5725191	5.2164352	0	47.0000000
WACC	262	0.7977099	1.4625316	0	9.0000000
DACC	262	1.3091603	2.1352344	0	18.0000000
XVOL	262	5.2083969	3.9509839	0.1000000	20.0000000
RVOL	262	5.5681679	4.3218418	0.0700000	21.9000000
FRATE	262	0.0061641	0.0583757	0	0.6090000
FIRATE	262	0.4221374	0.9271722	0	10.6500000
TRATE	262	0.9328244	1.3298309	0	13.7000000
WRATE	262	0.1669900	0.3475415	0	3.0400000
DRATE	262	0.2735564	0.4837937	0	4.5666667

Ramp Type= On

Urban/Rural =Urban DESCR=loopnoleft3

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	152	4.9605263	9.4110556	0	89.0000000
FACC	152	0.0131579	0.1143274	0	1.0000000
IACC	152	1.3750000	2.7855172	0	24.0000000
FIACC	152	1.3881579	2.7908302	0	24.0000000
MVACC	152	2.3815789	6.9224026	0	80.0000000
WACC	152	1.1842105	1.9203375	0	11.0000000
DACC	152	1.8618421	3.5918402	0	30.0000000
XVOL	152	6.7782895	7.2827922	0.2000000	50.0000000
RVOL	152	7.0621053	7.8777432	0.1600000	54.7500000
FRATE	152	0.0029671	0.0308677	0	0.3730000
FIRATE	152	0.1958553	0.3066880	0	1.7700000
TRATE	152	0.7202632	0.9224657	0	7.6600000
WRATE	152	0.2251360	0.4396908	0	3.5353846
DRATE	152	0.2560791	0.3689140	0	1.9114286

Ramp Type= On

Urban/Rural =Urban DESCR=loopnoleft4

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	581	3.6643718	4.4600820	0	36.0000000
FACC	581	0.0051635	0.0717336	0	1.0000000
IACC	581	1.2530120	2.0543508	0	20.0000000
FIACC	581	1.2581756	2.0549661	0	20.0000000
MVACC	581	2.4647160	3.5638333	0	36.0000000
WACC	581	0.9535284	1.8771518	0	20.0000000
DACC	581	0.9896730	1.5051280	0	9.0000000
XVOL	581	5.0271945	3.6544707	0	25.5000000
RVOL	581	5.3875387	4.0392559	0.0500000	27.9200000
FRATE	581	0.0013012	0.0234061	0	0.5370000
FIRATE	581	0.2849225	1.2115448	0	27.4000000
TRATE	581	0.7857659	2.0336718	0	45.6600000
WRATE	581	0.1770545	0.4859096	0	9.1320000
DRATE	581	0.2172759	0.5326046	0	9.1320000

Ramp Type= On

Urban/Rural =Urban DESCR=restarea

Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	39	1.1282051	2.7926547	0	16.0000000
FACC	39	0	0	0	0
IACC	39	0.0769231	0.3542684	0	2.0000000
FIACC	39	0.0769231	0.3542684	0	2.0000000
MVACC	39	0.5384615	1.1886780	0	6.0000000
WACC	39	0.0769231	0.3542684	0	2.0000000
DACC	39	0.4358974	1.5007870	0	9.0000000
XVOL	39	2.4179487	2.6511042	0	9.6000000
RVOL	39	2.6376923	2.9083459	0	10.5100000
FRATE	39	0	0	0	0
FIRATE	39	0.0189744	0.0827105	0	0.3800000
TRATE	39	0.7758974	2.7266354	0	16.5000000
WRATE	39	0.0159402	0.0710806	0	0.3766667
DRATE	39	0.1413942	0.4726420	0	2.7393750

Appendix B Descriptive Statistics on Individual Ramp Configurations

Ramp Type= On

Urban/Rural =Urban		DESCR=scissors			
Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	144	3.3402778	4.3903314	0	27.0000000
FACC	144	0	0	0	0
IACC	144	1.1388889	1.7644949	0	11.0000000
FIACC	144	1.1388889	1.7644949	0	11.0000000
MVACC	144	2.5347222	3.7823905	0	26.0000000
WACC	144	0.6388889	1.2381529	0	9.0000000
DACC	144	0.8750000	1.3320944	0	7.0000000
XVOL	144	6.0173611	5.2394994	0.4000000	33.0000000
RVOL	144	6.4695139	5.7468211	0.4400000	36.1400000
FRATE	144	0	0	0	0
FIRATE	144	0.2115278	0.4197724	0	3.6500000
TRATE	144	0.5910417	0.7650181	0	3.9600000
WRATE	144	0.1093061	0.2302294	0	1.2876923
DRATE	144	0.1697863	0.2799748	0	1.8250000

Ramp Type= On

Urban/Rural =Urban		DESCR=slip3			
Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	66	2.3030303	3.0833034	0	13.0000000
FACC	66	0.0303030	0.1727334	0	1.0000000
IACC	66	0.5606061	0.8966397	0	4.0000000
FIACC	66	0.5909091	0.9110817	0	4.0000000
MVACC	66	1.7424242	2.4388560	0	11.0000000
WACC	66	0.3787879	0.9074928	0	5.0000000
DACC	66	0.7121212	1.1471054	0	6.0000000
XVOL	66	14.2651515	13.2502017	0.1000000	46.3000000
RVOL	66	14.1034848	14.3114306	0.0800000	50.7000000
FRATE	66	0.0010455	0.0059754	0	0.0370000
FIRATE	66	0.2257576	1.1370502	0	9.0400000
TRATE	66	0.7713636	2.2527063	0	12.2900000
WRATE	66	0.0396120	0.1293264	0	0.9123077
DRATE	66	0.1119434	0.3600892	0	2.2807692

Ramp Type= On

Urban/Rural =Urban		DESCR=slip4			
Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	78	2.4487179	2.4844840	0	13.0000000
FACC	78	0.0128205	0.1132277	0	1.0000000
IACC	78	0.8205128	1.1018708	0	4.0000000
FIACC	78	0.8333333	1.0980108	0	4.0000000
MVACC	78	1.9358974	2.2409399	0	11.0000000
WACC	78	0.3333333	0.8477115	0	6.0000000
DACC	78	0.8589744	1.0778087	0	4.0000000
XVOL	78	8.9320513	8.4231359	0.1000000	50.0000000
RVOL	78	9.5170513	9.3052037	0.0400000	54.7500000
FRATE	78	0.0334615	0.2955243	0	2.6100000
FIRATE	78	0.5126923	2.6253983	0	22.6100000
TRATE	78	0.7679487	2.6881406	0	22.6100000
WRATE	78	0.3296317	2.5571140	0	22.6100000
DRATE	78	0.1299409	0.2261726	0	1.2800000

Ramp Type= On

Urban/Rural =Urban		DESCR=split			
Variable	N	Mean	Std Dev	Minimum	Maximum
TACC	343	4.0262391	6.9300523	0	76.0000000
FACC	343	0.0204082	0.1415985	0	1.0000000
IACC	343	1.2448980	1.9098303	0	15.0000000
FIACC	343	1.2653061	1.9367613	0	15.0000000
MVACC	343	2.7521866	5.7312970	0	70.0000000
WACC	343	0.8454810	1.6796081	0	15.0000000
DACC	343	1.4285714	2.6863310	0	29.0000000
XVOL	343	17.8084548	13.6049876	0.8000000	92.9000000
RVOL	343	18.3916910	14.3410589	0.9100000	82.3400000
FRATE	343	0.0024373	0.0259238	0	0.4490000
FIRATE	343	0.0854810	0.1675407	0	2.2000000
TRATE	343	0.2400875	0.3601945	0	4.4000000
WRATE	343	0.0599630	0.2038571	0	3.3000000
DRATE	343	0.0879887	0.2071668	0	3.3000000

Appendix C Analysis Variance and Covariance Models

Appendix C

Analysis Variance and Covariance Models (Detail Description)

This portion of the analyses included a series of ANOVA and ANCOVA models, again stratified by rural/urban and on/off-ramp, where the ramp traffic volume were specified as a covariate. The analyses were done after performing a linear adjustment for ramps traffic volume effect (ANCOVA). The rationale for doing this analysis was that previous studies indicated traffic volume as a strong predictor of accidents. A series of ANCOVA models were run where location of ramp (Caltrans district) was used as a predictor. The motivation for these latter analyses was to account for district differences could otherwise be attributed to differences in ramp volumes. In these later analyses, if the differences in ramps were significant, then pairwise comparisons from ANCOVA with district as predictor were selected.

While the ANOVA/ANCOVA can tell that a certain factor is influential, they don't tell the nature (direction) of any significant effects that are detected. For example, from ANOVA tables which will be discussed later in this report, we know that there are systematic differences between two ramp configuration, but we can not tell which of the two have higher overall accident rates. For this reason, the ANOVA/ANCOVA analyses were followed by *post hoc* analyses.

The post hoc analysis used for the pairwise comparisons is Tukey's⁴ method which exercises extra caution in declaring differences as significant.

⁴ Statistical comparisons ordinarily involve an inherent false positive error rate of 5%, this means that you would expect on the order of 10 false positive comparisons among the 210 comparisons. Since the purpose of this analysis is to identify differences according to ramp type with some degree of confidence, this high error rate is unacceptable. Tukey's method exercises extra caution in declaring differences as significant, and consequently the overall Type I error rate will be approximately 5%. By exercising this caution however, the likelihood of a false negative (Type II) error increases, and for this reason, it's important to remember that in the absence of a statistically significant difference between a particular pair of ramp types, any of the following explanations is plausible: (i) there in fact is no difference between the ramp types, (ii) there is a difference, which went undetected due to the conservatism of the *post hoc* analysis, (iii) there is a difference, which went undetected because the magnitude of the difference was small, or (iv) there is a difference, which went undetected because there were relatively few ramps for the two types being compared. In view of these four possibilities, it's appropriate that the distinction between a significant difference and an insignificant one should be interpreted as being between those differences for which there was sufficient evidence to declare a difference, and those for which there simply was insufficient evidence.

Appendix C Analysis Variance and Covariance Models

Since the pairwise comparison were done after adjustment for the covariate, comparisons method used was the *least square means*, (LSM). The results from LSM were printed as a matrix of p-values for comparing each pair of ramps.

In pairwise comparisons, the overall Type I error rate will be approximately 5%; in other words, the probability that any of the differences found to be statistically significant among the comparisons will be 5% or less. By exercising this caution, however, the likelihood of a false-negative (Type II) error increases. For this reason, it is important to remember that in the absence of a statistically significant difference between a particular pair of ramp types, any of the following explanations is plausible:

- there is no difference between the ramp types;
- there is a difference, which went undetected due to the conservatism of the *post hoc* analysis;
- there is a difference, which went undetected because the magnitude of the difference was small; or
- there is a difference, which went undetected because there were relatively few ramps for the two types being compared.

In view of these four possibilities, it is appropriate that the distinction between a significant difference and an insignificant one should be interpreted as being between those differences for which there is sufficient evidence to declare a difference, and those for which there simply is insufficient evidence.

In the *post hoc* results, many of the ramp types that are comparatively rare fail to be significantly different from any of the other ramp types. All this means is that there is too little information available on those ramp types to draw meaningful conclusions.

Interpretation of ANOVA/ANCOVA Results

In reading the ANOVA/ANCOVA tables, the F-values reflect the size of a systematic effect. The p-value is used to determine whether the effect in question is statistically significant. In other words, the p-value represents the probability that the result in question could have occurred by chance alone, and it's customary to ascribe statistical significance to any tests with p-values less than or equal to 0.05. The smaller a p-value, the less likely a result could have occurred simply by chance. The following analyses consists of two sets of ANCOVA models.

- ANCOVA for stratified data by on/off, urban/rural, with adjustment for traffic volume
- ANCOVA for stratified data by on/off, urban/rural, with adjustment for traffic volume and district as predictor.

Appendix C Analysis Variance and Covariance Models

Each of the two models were run with the following four data sets:

- ONOFF=off RURAL=r----->(off-ramps in rural area)
- ONOFF=off RURAL=u----->(off-ramps in urban area)
- ONOFF=on RURAL=r----->(on-ramps in rural area)
- ONOFF=on RURAL=u----->(on-ramps in urban area)

For each of the above eight cases, mean accident rate tables and p-value matrix is provided for four accident rate types (total, fatal+injury, wet, and dark accident rates). Because the ANCOVA analyses indicated district effect as being significant (meaning some differences attributed to Caltrans district and not just to ramp volumes) in urban area, comparison tables for urban settings were presented from ANCOVA models that modeled district as predictor. The tables for rural settings are from ANCOVA models without district as predictor.

Comparison of Accident Rates by Type of Ramp Stratified by on/off, urban/rural, Adjusting for Traffic Volume

General Linear Models Procedure
Class Level Information

Class Levels Values
DESCR 14 buttonhook diamond4 diamond4 dir/semi3 dir/semi4 direct loopleft3 loopleft4 loopnoleft3 loopnoleft4 restarea
scissors sltpt split

Number of observations in by group : 1368

Dependent Variable: **PIRATE**

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	14	400.84198626	28.63157045	2.18	0.0070
Error	1350	17758.53283308	13.15446877		
Corrected Total	1364	18159.37481934			
	R-Square	C.V.	Root MSE		PIRATE Mean
	0.022074	485.2659	3.62690898		0.74740659
Source	DF	Type I SS	Mean Square	F Value	Pr > F
DESCR	13	340.37527522	26.18271348	1.99	0.0185
RVOL	1	60.46671103	60.46671103	4.60	0.0322
Source	DF	Type III SS	Mean Square	F Value	Pr > F
DESCR	13	310.85585084	23.91198853	1.82	0.0358
RVOL	1	60.46671103	60.46671103	4.60	0.0322

Dependent Variable: **TRATE**

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	14	1358.85731752	97.06123697	3.13	0.0001
Error	1350	41822.42328248	30.97957280		
Corrected Total	1364	43181.28060000			
	R-Square	C.V.	Root MSE		TRATE Mean
	0.031469	297.6433	5.56592964		1.87000300
Source	DF	Type I SS	Mean Square	F Value	Pr > F
DESCR	13	1056.96651260	81.30511635	2.62	0.0013
RVOL	1	301.89080492	301.89080492	9.74	0.0018
Source	DF	Type III SS	Mean Square	F Value	Pr > F
DESCR	13	926.26541013	71.25118539	2.30	0.0052
RVOL	1	301.89080492	301.89080492	9.74	0.0018

Dependent Variable: **WRATE**

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	14	6.87386262	0.49099019	0.67	0.8020
Error	1350	984.11029018	0.72897059		
Corrected Total	1364	990.98415279			
	R-Square	C.V.	Root MSE		WRATE Mean
	0.006916	596.8256	0.85379774		0.14305650
Source	DF	Type I SS	Mean Square	F Value	Pr > F
DESCR	13	6.67198867	0.51322990	0.70	0.7608
RVOL	1	0.20187395	0.20187395	0.28	0.5988
Source	DF	Type III SS	Mean Square	F Value	Pr > F
DESCR	13	5.89090530	0.45314656	0.62	0.8377
RVOL	1	0.20187395	0.20187395	0.28	0.5988

Dependent Variable: **DRATE**

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	14	169.81452327	12.13103738	1.36	0.1674
Error	1350	12076.58894263	8.94562144		
Corrected Total	1364	12246.42346591			
	R-Square	C.V.	Root MSE		DRATE Mean
	0.013868	422.8695	2.99092318		0.70729217
Source	DF	Type I SS	Mean Square	F Value	Pr > F
DESCR	13	99.52629849	7.6586911	0.86	0.6003
RVOL	1	70.30822479	70.30822479	7.86	0.0051
Source	DF	Type III SS	Mean Square	F Value	Pr > F
DESCR	13	83.69801995	6.43830923	0.72	0.7451
RVOL	1	70.30822479	70.30822479	7.86	0.0051

Least Squares Means
Adjustment for multiple comparisons: Tukey-Kramer

DESCR	LSMEAN	Number
buttonhook	1.62399444	1
diamond3	0.21383081	2
diamond4	0.74358644	3
dir/semi3	0.97714972	4
dir/semi4	0.52728234	5
direct	0.45588443	6
loopleft3	0.81766351	7
loopleft4	0.86071078	8
loopnoleft3	0.25061523	9
loopnoleft4	4.43429690	10
restarea	0.16719093	11
scissors	0.61611882	12
slip4	-0.13165370	13
split	0.85029422	14

FIRATE Pr > |T| H0: LSMEAN(i)=LSMEAN(j)

i/j	1	2	3	4	5	6	7	8	9	10	11	12	13	14
1	.	0.2512	0.4783	0.9996	0.8968	0.9917	1.0000	0.9996	0.9882	0.4433	0.1269	0.9742	1.0000	1.0000
2	0.2512	.	0.9888	0.9983	1.0000	1.0000	1.0000	1.0000	1.0000	0.0195	1.0000	1.0000	1.0000	1.0000
3	0.4783	0.9888	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.0511	0.9416	1.0000	1.0000	1.0000
4	0.9996	0.9983	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	0.2182	0.9956	1.0000	1.0000	1.0000
5	0.8968	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	0.0759	1.0000	1.0000	1.0000	1.0000
6	0.9917	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	0.1867	1.0000	1.0000	1.0000	1.0000
7	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	0.9133	1.0000	1.0000	1.0000	1.0000
8	0.9996	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	0.2672	0.9999	1.0000	1.0000	1.0000
9	0.9882	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	0.1956	1.0000	1.0000	1.0000	1.0000
10	0.4433	0.0195	0.0511	0.2182	0.0759	0.1867	0.9133	0.2672	0.1956	.	0.0143	1.0000	1.0000	0.9924
11	0.1269	1.0000	0.9416	0.9956	1.0000	1.0000	1.0000	0.9999	1.0000	0.0143	.	1.0000	1.0000	1.0000
12	0.9742	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.1185	1.0000	.	1.0000	1.0000
13	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9955	1.0000	1.0000	.	1.0000
14	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9924	1.0000	1.0000	1.0000	1.0000	.

Dependent Variable: **TRATE**

buttonhook	4.02667877	1
diamond3	0.75819771	2
diamond4	1.66200593	3
dir/semi3	1.70607294	4
dir/semi4	1.25042281	5
direct	1.02927861	6
loopleft3	5.93395625	7
loopleft4	3.58284530	8
loopnoleft3	1.23191103	9
loopnoleft4	4.44613141	10
restarea	2.17379991	11
scissors	1.32859920	12
slip4	-0.29863984	13
split	1.80734072	14

Dependent Variable: **TRATE**

i/j	1	2	3	4	5	6	7	8	9	10	11	12	13	14
1	.	0.0028	0.0022	0.6277	0.1935	0.6679	1.0000	1.0000	0.8893	1.0000	0.3974	0.3765	1.0000	1.0000
2	0.0028	.	0.9719	0.9998	1.0000	1.0000	0.9231	0.5924	1.0000	0.7184	0.8614	1.0000	1.0000	1.0000
3	0.0022	0.9719	.	1.0000	1.0000	1.0000	0.9833	0.9182	1.0000	0.9329	0.9997	1.0000	1.0000	1.0000
4	0.6277	0.9998	1.0000	.	1.0000	1.0000	0.9903	0.9909	1.0000	0.9779	1.0000	1.0000	1.0000	1.0000
5	0.1935	1.0000	1.0000	1.0000	.	1.0000	0.9713	0.9154	1.0000	0.9100	0.9995	1.0000	1.0000	1.0000
6	0.6679	1.0000	1.0000	1.0000	1.0000	.	0.9730	0.9674	1.0000	0.9429	0.9999	1.0000	1.0000	1.0000
7	1.0000	0.9231	0.9833	0.9903	0.9713	0.9730	.	1.0000	0.9858	1.0000	0.9961	0.9777	0.9997	1.0000
8	1.0000	0.5924	0.9162	0.9900	0.9150	0.9674	1.0000	.	0.9928	1.0000	0.9971	0.9549	1.0000	1.0000
9	0.8893	1.0000	1.0000	1.0000	1.0000	1.0000	0.9858	0.9928	.	0.9730	1.0000	1.0000	1.0000	1.0000
10	1.0000	0.7184	0.9329	0.9779	0.9100	0.9429	1.0000	1.0000	0.9788	.	0.9911	0.9384	0.9999	1.0000
11	0.3974	0.8614	0.9997	1.0000	0.9995	0.9999	0.9961	0.9971	1.0000	0.9911	.	0.9999	1.0000	1.0000
12	0.3765	1.0000	1.0000	1.0000	1.0000	1.0000	0.9777	0.9549	1.0000	0.9384	0.9999	.	1.0000	1.0000
13	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9997	1.0000	1.0000	0.9999	1.0000	1.0000	.	1.0000
14	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.

Dependent Variable: **WRATE**

DESCR	WRATE LSMEAN	LSMEAN Number
buttonhook	0.12432404	1
diamond3	0.02021938	2
diamond4	0.19205276	3
dir/semi3	0.01860279	4
dir/semi4	0.07505332	5
direct	0.04772570	6
loopleft3	0.27002731	7
loopleft4	0.05439356	8
loopnoleft3	0.09123161	9
loopnoleft4	0.05140768	10
restarea	0.04700795	11
scissors	0.07286722	12
slip4	-0.00772259	13
split	0.08229696	14

Dependent Variable: **WRATE**

i/j	1	2	3	4	5	6	7	8	9	10	11	12	13	14
1		0.9999	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
2	0.9999		0.8633	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
3	1.0000	0.8633		0.9953	0.9997	1.0000	1.0000	0.9999	1.0000	1.0000	0.9054	0.9999	1.0000	1.0000
4	1.0000	1.0000	0.9953		1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
5	1.0000	1.0000	0.9997	1.0000		1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
6	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
7	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
8	1.0000	1.0000	0.9999	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
9	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000	1.0000	1.0000	1.0000
10	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000	1.0000	1.0000
11	1.0000	1.0000	0.9054	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000	1.0000
12	1.0000	1.0000	0.9999	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000
13	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000
14	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	

Dependent Variable: **DRATE**

Adjustment for multiple comparisons: Tukey-Kramer

DESCR	DRATE LSMEAN	LSMEAN Number
buttonhook	1.27951525	1
diamond3	0.30755313	2
diamond4	0.66029609	3
dir/semi3	0.70058109	4
dir/semi4	0.58233594	5
direct	0.41991636	6
loopleft3	1.69927773	7
loopleft4	0.31770178	8
loopnoleft3	0.63870128	9
loopnoleft4	-0.01471671	10
restarea	1.05371337	11
scissors	0.67157370	12
slip4	-0.14412054	13
split	0.43600993	14

Dependent Variable: **DRATE**

i/j	1	2	3	4	5	6	7	8	9	10	11	12	13	14
1		0.5557	0.7356	0.9991	0.9869	0.9972	1.0000	0.9754	1.0000	0.9852	1.0000	0.9987	1.0000	1.0000
2	0.5557		0.9986	1.0000	1.0000	1.0000	0.9999	1.0000	1.0000	1.0000	0.8776	1.0000	1.0000	1.0000
3	0.7356	0.9986		1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9879	1.0000	1.0000	1.0000
4	0.9991	1.0000	1.0000		1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
5	0.9869	1.0000	1.0000	1.0000		1.0000	1.0000	1.0000	1.0000	1.0000	0.9997	1.0000	1.0000	1.0000
6	0.9972	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000	1.0000	1.0000	0.9999	1.0000	1.0000	1.0000
7	1.0000	0.9998	1.0000	1.0000	1.0000	1.0000		0.9999	1.0000	0.9994	1.0000	1.0000	1.0000	1.0000
8	0.9754	1.0000	1.0000	1.0000	1.0000	1.0000	0.9999		1.0000	1.0000	0.9978	1.0000	1.0000	1.0000
9	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000	1.0000	1.0000	1.0000
10	0.9852	1.0000	1.0000	1.0000	1.0000	1.0000	0.9994	1.0000	1.0000		0.9976	1.0000	1.0000	1.0000
11	1.0000	0.8776	0.9879	1.0000	0.9997	0.9999	1.0000	0.9978	1.0000	0.9976		1.0000	1.0000	1.0000
12	0.9987	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000
13	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000
14	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	

Comparison of accident rates by type of ramp stratified by on/off, urban/rural, adjusting for traffic volume

Class Level Information

Class	Levels	Values
DESCR	14	butcorhook diamond3 diamond4 dir/semi3 dir/semi4 direct loopleft3 loopleft4 loopnoleft3 loopnoleft4 restarea scissors slip4 split

Number of observations in by group = 1367

NOTE: All dependent variable are consistent with respect to the presence or absence of missing values. However only 1364 observations can be used in this analysis.

Dependent Variable: **FIRATE**

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	14	81.78260263	5.98447162	0.75	0.7264
Error	1349	10792.16917450	8.00012541		
Corrected Total	1363	10875.95177713			
	R-Square	C.V.	Root MSE		FIRATE Mean
	0.007703	799.6859	2.82844929		0.35369501

Source	DF	Type I SS	Mean Square	F Value	Pr > F
DESCR	13	54.27620398	4.17509261	0.52	0.9124
RVOL	1	29.50639865	29.50639865	3.69	0.0550
Source	DF <td>Type III SS <td>Mean Square <td>F Value <td>Pr > F</td> </td></td></td>	Type III SS <td>Mean Square <td>F Value <td>Pr > F</td> </td></td>	Mean Square <td>F Value <td>Pr > F</td> </td>	F Value <td>Pr > F</td>	Pr > F
DESCR	13	61.18945453	4.70688112	0.59	0.8631
RVOL	1	29.50639865	29.50639865	3.69	0.0550

Dependent Variable: **TRATE**

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	14	188.86458016	13.49032715	0.79	0.6814
Error	1349	23046.82544001	17.08437764		
Corrected Total	1363	23235.69002016			
	R-Square	C.V.	Root MSE		TRATE Mean
	0.008128	472.2495	4.13332525		0.67524194

Source	DF	Type I SS	Mean Square	F Value	Pr > F
DESCR	13	82.17873622	6.32144125	0.37	0.9789
RVOL	1	106.68584393	106.68584393	6.24	0.0126
Source	DF <td>Type III SS <td>Mean Square <td>F Value <td>Pr > F</td> </td></td></td>	Type III SS <td>Mean Square <td>F Value <td>Pr > F</td> </td></td>	Mean Square <td>F Value <td>Pr > F</td> </td>	F Value <td>Pr > F</td>	Pr > F
DESCR	13	86.51599703	6.65507669	0.35	0.9735
RVOL	1	106.68584393	106.68584393	6.24	0.0126

Dependent Variable: **WRATE**

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	14	4.51064791	0.32361764	0.30	0.9937
Error	1349	1437.32527182	1.06547463		
Corrected Total	1363	1441.85591881			
	R-Square	C.V.	Root MSE		WRATE Mean
	0.003142	1024.073	1.03221830		0.10079536

Source	DF	Type I SS	Mean Square	F Value	Pr > F
DESCR	13	3.86139897	0.29703069	0.28	0.9945
RVOL	1	0.66924804	0.66924804	0.63	0.4282
Source	DF <td>Type III SS <td>Mean Square <td>F Value <td>Pr > F</td> </td></td></td>	Type III SS <td>Mean Square <td>F Value <td>Pr > F</td> </td></td>	Mean Square <td>F Value <td>Pr > F</td> </td>	F Value <td>Pr > F</td>	Pr > F
DESCR	13	3.62350045	0.27873080	0.26	0.9960
RVOL	1	0.66924804	0.66924804	0.63	0.4282

Dependent Variable: **DRATE**

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	14	87.13382083	6.22384434	0.75	0.7270
Error	1349	11231.88600992	8.32608303		
Corrected Total	1363	11319.01983075			
	R-Square	C.V.	Root MSE		DRATE Mean
	0.007698	917.8869	2.88549528		0.31436284

Source	DF	Type I SS	Mean Square	F Value	Pr > F
DESCR	13	67.01711204	5.15516401	0.62	0.8398
RVOL	1	20.11668799	20.11668799	2.42	0.1203
Source	DF <td>Type III SS <td>Mean Square <td>F Value <td>Pr > F</td> </td></td></td>	Type III SS <td>Mean Square <td>F Value <td>Pr > F</td> </td></td>	Mean Square <td>F Value <td>Pr > F</td> </td>	F Value <td>Pr > F</td>	Pr > F
DESCR	13	70.57697110	5.42899778	0.65	0.8107
RVOL	1	20.11668799	20.11668799	2.42	0.1203

Appendix C Analysis of On-Ramps in Rural Area

Dependent Variable: **FIRATE**

DESCR	FIRATE LSMEAN	LSMEAN Number
buttonhook	0.24334023	1
diamond3	0.21779679	2
diamond4	0.25615367	3
dir/semi3	0.60043056	4
dir/semi4	0.88943680	5
direct	0.52784949	6
loopleft3	0.00197261	7
loopleft4	0.48037372	8
loopnoleft3	0.17543346	9
loopnoleft4	0.35627676	10
restarea	0.81383110	11
scissors	0.08020731	12
slip4	1.06415180	13
split	0.37906062	14

Dependent Variable: **FIRATE**

i/j	1	2	3	4	5	6	7	8	9	10	11	12	13	14
1	.	1.0000	1.0000	1.0000	0.9747	1.0000	1.0000	1.0000	1.0000	1.0000	0.9648	1.0000	1.0000	1.0000
2	1.0000	.	1.0000	1.0000	0.9804	1.0000	1.0000	1.0000	1.0000	1.0000	0.9765	1.0000	1.0000	1.0000
3	1.0000	1.0000	.	1.0000	0.9058	1.0000	1.0000	1.0000	1.0000	1.0000	0.7785	1.0000	1.0000	1.0000
4	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9999	1.0000	1.0000
5	0.9747	0.9804	0.9058	1.0000	.	1.0000	1.0000	0.9999	0.9999	0.9999	1.0000	0.9844	1.0000	1.0000
6	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
7	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
8	1.0000	1.0000	1.0000	1.0000	0.9999	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
9	1.0000	1.0000	1.0000	1.0000	0.9993	1.0000	1.0000	1.0000	.	1.0000	0.9996	1.0000	1.0000	1.0000
10	1.0000	1.0000	1.0000	1.0000	1.0000	0.9998	1.0000	1.0000	1.0000	.	0.9999	1.0000	1.0000	1.0000
11	0.9648	0.9765	0.7785	1.0000	1.0000	1.0000	1.0000	0.9996	0.9999	0.9999	.	0.9859	1.0000	1.0000
12	1.0000	1.0000	1.0000	0.9999	0.9844	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000
13	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000
14	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.

Dependent Variable: **TRATE**

DESCR	TRATE LSMEAN	LSMEAN Number
buttonhook	1.17163388	1
diamond3	0.53195115	2
diamond4	0.76081578	3
dir/semi3	0.91388930	4
dir/semi4	1.12723866	5
direct	1.13654330	6
loopleft3	0.65660806	7
loopleft4	0.84025707	8
loopnoleft3	0.88602757	9
loopnoleft4	0.62430910	10
restarea	1.51773442	11
scissors	0.70342655	12
slip4	2.02347983	13
split	0.77744872	14

Dependent Variable: **TRATE**

i/j	1	2	3	4	5	6	7	8	9	10	11	12	13	14
1	.	0.9990	0.9995	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
2	0.9990	.	1.0000	1.0000	0.9999	1.0000	1.0000	1.0000	1.0000	1.0000	0.9370	1.0000	1.0000	1.0000
3	0.9995	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.8568	1.0000	1.0000	1.0000
4	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9999	1.0000	1.0000	1.0000
5	1.0000	0.9999	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
6	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
7	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
8	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	0.9983	1.0000	1.0000	1.0000
9	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000
10	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	0.9980	1.0000	1.0000	1.0000
11	1.0000	0.9370	0.8568	0.9999	1.0000	1.0000	1.0000	0.9983	1.0000	0.9980	.	0.9990	1.0000	1.0000
12	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000
13	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000
14	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.

Appendix C Analysis of On-Ramps in Rural Area

Dependent Variable: **WRATE**

DESCR	WRATE LSMEAN	LSMEAN Number
buttonhook	0.06118293	1
diamond3	0.04237459	2
diamond4	0.11047215	3
dir/semi3	0.08443759	4
dir/semi4	0.09041774	5
direct	0.09058624	6
loopleft3	0.00029708	7
loopleft4	0.06248182	8
loopnoleft3	0.45479040	9
loopnoleft4	0.16713453	10
restarea	0.09134084	11
scissors	0.04160251	12
slip4	0.16026513	13
split	0.11375457	14

Dependent Variable: **WRATE**

i/j	Pr > T HO: LSMEAN(i)=LSMEAN(j)													
	1	2	3	4	5	6	7	8	9	10	11	12	13	14
1	.													
2	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9464	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
3	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	0.9395	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
4	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	0.9666	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
5	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	0.9882	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
6	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	0.9813	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
7	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	0.9981	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
8	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	0.9993	1.0000	1.0000	1.0000	1.0000	1.0000
9	0.9464	0.9395	0.9666	0.9882	0.9813	0.9981	0.9993	0.9617	.	0.9993	0.9700	0.9745	1.0000	1.0000
10	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9993	0.9700	.	1.0000	1.0000	1.0000	1.0000
11	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9700	1.0000	1.0000	.	1.0000	1.0000	1.0000
12	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9745	1.0000	1.0000	1.0000	.	1.0000	1.0000
13	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000
14	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.

Dependent Variable: **DRATE**

DESCR	DRATE LSMEAN	LSMEAN Number
buttonhook	0.59826309	1
diamond3	0.17449399	2
diamond4	0.24253680	3
dir/semi3	0.49783010	4
dir/semi4	0.12922160	5
direct	0.47333289	6
loopleft3	0.00162878	7
loopleft4	0.07614066	8
loopnoleft3	0.02930553	9
loopnoleft4	0.22117764	10
restarea	0.92455572	11
scissors	0.08074969	12
slip4	0.87866580	13
split	0.31298880	14

Dependent Variable: **DRATE**

i/j	Pr > T HO: LSMEAN(i)=LSMEAN(j)													
	1	2	3	4	5	6	7	8	9	10	11	12	13	14
1	.													
2	0.9994	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9990	0.9999	1.0000	0.9999	0.9997	1.0000	1.0000
3	0.9956	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9999	0.9999	1.0000	1.0000
4	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9999	1.0000	1.0000	1.0000
5	0.9990	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	0.9999	1.0000	1.0000	1.0000
6	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
7	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
8	0.9960	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
9	0.9999	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000
10	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	0.9936	1.0000	1.0000	1.0000
11	0.9999	0.8865	0.4977	0.9999	0.8811	1.0000	0.9999	0.7891	0.9901	0.9936	.	0.9610	1.0000	1.0000
12	0.9997	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9610	.	1.0000	1.0000
13	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000
14	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.

Comparison of accident rates by ramp configuration, traffic volume as covariate, and district= predictor

General Linear Models Procedure
Class Level Information

Class	Levels	Values
DISTRICT	12	1 2 3 4 5 6 7 8 9 10 11 12
DESCR	15	buttonhook diamond3 diamond4 dir/semi3 dir/semi4 direct loopleft3 loopleft4 loopnoleft3 loopnoleft4 restarea scissors slip3 slip4 split

Number of observations in by group = 5309

NOTE: All dependent variable are consistent with respect to the presence or absence of missing values. However only 5107 observations can be used in this analysis.

Dependent Variable: **FIRATE**

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	26	475.84036167	18.30155237	5.72	0.0001
Error	5280	16901.52131253	3.20104570		
Corrected Total	5306	17377.36167420			
	R-Square	C.V.	Root MSE	FIRATE Mean	
	0.027383	334.4276	1.78914664	0.53498775	

Source	DF	Type I SS	Mean Square	F Value	Pr > F
DISTRICT	11	153.89012730	13.99001157	4.37	0.0001
RVOL	1	155.28265817	155.28265817	48.51	0.0001
DESCR	14	166.66757620	11.90482687	3.72	0.0001

Source	DF	Type III SS	Mean Square	F Value	Pr > F
DISTRICT	11	104.25870264	9.47886388	2.96	0.0006
RVOL	1	51.01326659	51.01326659	15.94	0.0001
DESCR	14	166.66757620	11.90482687	3.72	0.0001

Dependent Variable: **TRATE**

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	36	3940.39630420	108.33205444	5.42	0.0001
Error	5280	147518.83717769	27.93917371		
Corrected Total	5306	151459.23348189			
	R-Square	C.V.	Root MSE	TRATE Mean	
	0.026016	379.6888	5.28575195	1.39212738	

Source	DF	Type I SS	Mean Square	F Value	Pr > F
DISTRICT	11	1941.00866803	176.45533346	6.32	0.0001
RVOL	1	1042.60099487	1042.60099487	37.32	0.0001
DESCR	14	956.78664130	68.34190295	2.45	0.0019

Source	DF	Type III SS	Mean Square	F Value	Pr > F
DISTRICT	11	1414.06659190	128.55150835	4.60	0.0001
RVOL	1	377.48531314	377.48531314	13.51	0.0002
DESCR	14	956.78664130	68.34190295	2.45	0.0019

Dependent Variable: **WRATE**

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	26	83.32227668	3.20470295	3.18	0.0001
Error	5280	5323.27820217	1.00819663		
Corrected Total	5306	5406.60047885			
	R-Square	C.V.	Root MSE	WRATE Mean	
	0.015411	510.4535	1.00406995	0.19670548	

Source	DF	Type I SS	Mean Square	F Value	Pr > F
DISTRICT	11	48.74671622	4.43151966	4.40	0.0001
RVOL	1	12.92978785	12.92978785	12.82	0.0003
DESCR	14	21.64577262	1.54612662	1.53	0.0906

Source	DF	Type III SS	Mean Square	F Value	Pr > F
DISTRICT	11	40.92160724	3.72014611	3.69	0.0001
RVOL	1	2.42605006	2.42605006	2.41	0.1209
DESCR	14	21.64577262	1.54612662	1.53	0.0906

Dependent Variable: **DRATE**

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	26	329.56005941	12.67538690	3.47	0.0001
Error	5280	19297.14089122	3.65476153		
Corrected Total	5306	19626.70095064			
	R-Square	C.V.	Root MSE	DRATE Mean	
	0.016791	451.7622	1.91174306	0.42317462	

Source	DF	Type I SS	Mean Square	F Value	Pr > F
DISTRICT	11	152.94477134	13.90407012	3.80	0.0001
RVOL	1	100.64803479	100.64803479	27.54	0.0001
DESCR	14	75.96725328	5.42623238	1.48	0.1077

Source	DF	Type III SS	Mean Square	F Value	Pr > F
DISTRICT	11	111.44055022	10.13095911	2.77	0.0014
RVOL	1	35.67477867	35.67477867	9.76	0.0018
DESCR	14	75.96725328	5.42623238	1.48	0.1077

Dependent Variable: **FIRATE**

General Linear Models Procedure
Least Squares Means
Adjustment for multiple comparisons: Tukey-Kramer

DESCR	FIRATE LSMEAN	LSMEAN Number
buttonhook	0.50341603	1
diamond3	0.42852379	2
diamond4	0.72351217	3
dir/semi3	0.24637697	4
dir/semi4	0.39873186	5
direct	0.39279325	6
loopleft3	0.30723402	7
loopleft4	0.68379428	8
loopnoleft3	0.29283242	9
loopnoleft4	0.43639101	10
restarea	0.39750200	11
scissors	0.43147053	12
slip3	0.44117279	13
slip4	0.21072101	14
split	0.25721170	15

Dependent Variable: **FIRATE**

i/j	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
1	.	1.0000	0.4690	0.7818	0.9999	1.0000	1.0000	0.9980	0.9975	1.0000	1.0000	1.0000	1.0000	0.9814	0.7789
2	1.0000	.	0.7964	0.9993	1.0000	1.0000	1.0000	0.9932	1.0000	1.0000	1.0000	1.0000	1.0000	0.9998	0.9995
3	0.4690	0.7964	.	0.0005	0.0752	0.3111	1.0000	1.0000	0.3269	0.5123	0.9985	0.8854	0.9901	0.2838	0.0001
4	0.7818	0.9993	0.0005	.	0.9984	0.9997	1.0000	0.3209	1.0000	0.9948	1.0000	0.9996	0.9999	1.0000	1.0000
5	0.9999	1.0000	0.0752	0.9984	.	1.0000	1.0000	0.9070	1.0000	1.0000	1.0000	1.0000	1.0000	0.9999	0.9988
6	1.0000	1.0000	0.3111	0.9997	1.0000	.	1.0000	0.9471	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9997
7	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
8	0.9980	0.9932	1.0000	0.3209	0.9070	0.9471	1.0000	.	0.8488	0.9849	0.9999	0.9961	0.9996	0.7396	0.3161
9	0.9975	1.0000	0.3269	1.0000	1.0000	1.0000	0.8488	0.9849	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
10	1.0000	1.0000	0.5123	0.9948	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	0.9994	0.9959
11	1.0000	1.0000	0.9985	1.0000	1.0000	1.0000	0.9999	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000
12	1.0000	1.0000	0.8854	0.9996	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	0.9999	0.9997
13	1.0000	1.0000	0.9901	0.9999	1.0000	1.0000	1.0000	0.9996	1.0000	1.0000	1.0000	1.0000	.	1.0000	0.9999
14	0.9814	0.9998	0.2838	1.0000	0.9999	1.0000	1.0000	0.7396	1.0000	0.9994	1.0000	0.9999	1.0000	.	1.0000
15	0.7789	0.9995	0.0001	1.0000	0.9988	0.9997	1.0000	0.3161	1.0000	0.9959	1.0000	0.9997	0.9999	1.0000	.

Dependent Variable: **TRATE**

DESCR	TRATE LSMEAN	LSMEAN Number
buttonhook	1.43518009	1
diamond3	0.92078871	2
diamond4	1.80765971	3
dir/semi3	0.63320863	4
dir/semi4	1.02215120	5
direct	1.35150340	6
loopleft3	0.76486285	7
loopleft4	1.53141051	8
loopnoleft3	0.82078505	9
loopnoleft4	1.19714423	10
restarea	1.75940474	11
scissors	1.09597585	12
slip3	0.76406084	13
slip4	0.55398667	14
split	0.71747941	15

Dependent Variable: **TRATE**

i/j	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
1	.	0.9993	0.9866	0.7066	0.9993	1.0000	1.0000	1.0000	0.9978	1.0000	1.0000	1.0000	0.9995	0.9780	0.7954
2	0.9993	.	0.7747	1.0000	1.0000	1.0000	1.0000	0.9993	1.0000	1.0000	0.9999	1.0000	1.0000	1.0000	1.0000
3	0.9866	0.7747	.	0.0114	0.3248	0.9957	1.0000	1.0000	0.7570	0.9241	1.0000	0.9749	0.9310	0.6183	0.0112
4	0.7066	1.0000	0.0114	.	0.9997	0.9470	1.0000	0.8733	1.0000	0.9946	0.9952	0.9999	1.0000	1.0000	1.0000
5	0.9993	1.0000	0.3248	0.9997	.	1.0000	1.0000	0.9992	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
6	1.0000	1.0000	0.9957	0.9470	1.0000	.	1.0000	1.0000	0.9959	1.0000	1.0000	1.0000	0.9999	0.9959	0.9681
7	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
8	1.0000	0.9993	1.0000	0.8733	0.9992	1.0000	1.0000	.	0.9979	1.0000	1.0000	1.0000	0.9992	0.9810	0.9233
9	0.9978	1.0000	0.7570	1.0000	1.0000	1.0000	0.9979	0.9979	.	1.0000	0.9997	1.0000	1.0000	1.0000	1.0000
10	1.0000	1.0000	0.9241	0.9946	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	0.9996	0.9986
11	1.0000	0.9999	1.0000	0.9952	1.0000	1.0000	1.0000	0.9997	1.0000	1.0000	.	1.0000	0.9998	0.9971	0.9977
12	1.0000	1.0000	0.9749	0.9999	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000
13	0.9995	1.0000	0.9310	1.0000	1.0000	0.9999	1.0000	0.9992	1.0000	1.0000	0.9998	1.0000	.	1.0000	1.0000
14	0.9780	1.0000	0.6183	1.0000	1.0000	0.9959	1.0000	0.9810	1.0000	0.9996	0.9971	1.0000	1.0000	.	1.0000
15	0.7954	1.0000	0.0112	1.0000	1.0000	0.9681	1.0000	0.9233	1.0000	0.9986	0.9977	1.0000	1.0000	1.0000	.

Dependent Variable: **WRATE**

DESCR	WRATE LSMEAN	LSMEAN Number
buttonhook	0.2429221	1
diamond3	0.12550320	2
diamond4	0.21872160	3
dir/semi3	0.08546999	4
dir/semi4	0.10178322	5
direct	0.14797564	6
loopleft3	0.12609693	7
loopleft4	0.16180246	8
loopnoleft3	0.17119149	9
loopnoleft4	0.16392787	10
restarea	0.23802129	11
scissors	0.18433324	12
slip3	0.06581891	13
slip4	0.08481518	14
split	0.05931988	15

Dependent Variable: **WRATE**

Pr > |T| H0: LSMEAN(i)=LSMEAN(j)

i/j	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
1	.	0.9948	1.0000	0.6543	0.7593	0.9976	1.0000	0.9998	1.0000	0.9996	1.0000	1.0000	0.9835	0.9869	0.3037
2	0.9948	.	0.9898	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
3	1.0000	0.9898	.	0.3535	0.4782	0.9932	1.0000	0.9986	1.0000	0.9988	1.0000	1.0000	0.9778	0.9812	0.0637
4	0.6543	1.0000	0.3535	.	1.0000	1.0000	1.0000	1.0000	1.0000	0.9998	0.9999	0.9998	1.0000	1.0000	1.0000
5	0.7593	1.0000	0.4782	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
6	0.9976	1.0000	0.9932	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9984
7	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
8	0.9998	1.0000	0.9986	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9983
9	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	0.9988
10	0.9996	1.0000	0.9988	0.9998	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	0.9939
11	1.0000	1.0000	1.0000	0.9999	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	0.9999	1.0000	0.9993
12	1.0000	1.0000	1.0000	0.9998	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	0.9959
13	0.9835	1.0000	0.9778	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9999	1.0000	1.0000	.	1.0000	1.0000
14	0.9869	1.0000	0.9812	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000
15	0.3037	1.0000	0.0637	1.0000	1.0000	0.9984	1.0000	0.9983	0.9988	0.9939	0.9993	0.9959	1.0000	1.0000	.

Dependent Variable: **DRATE**

DESCR	DRATE LSMEAN	LSMEAN Number
buttonhook	0.50559134	1
diamond3	0.22101448	2
diamond4	0.50145992	3
dir/semi3	0.22855588	4
dir/semi4	0.30414788	5
direct	0.29588247	6
loopleft3	0.24681041	7
loopleft4	0.54585430	8
loopnoleft3	0.28805515	9
loopnoleft4	0.32265925	10
restarea	0.79293064	11
scissors	0.42416136	12
slip3	0.26587741	13
slip4	0.14498643	14
split	0.22128444	15

Dependent Variable: **DRATE**

Pr > |T| H0: LSMEAN(i)=LSMEAN(j)

i/j	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
1	.	0.9325	1.0000	0.7707	0.9693	0.9895	1.0000	1.0000	0.9981	0.9964	0.9999	1.0000	0.9995	0.9382	0.6674
2	0.9325	.	0.9052	1.0000	1.0000	1.0000	1.0000	0.9657	1.0000	1.0000	0.9377	0.9999	1.0000	1.0000	1.0000
3	1.0000	0.9052	.	0.4740	0.8759	0.9640	1.0000	1.0000	0.9964	0.9874	0.9998	1.0000	0.9993	0.9067	0.2935
4	0.7707	1.0000	0.4740	.	1.0000	1.0000	1.0000	0.8922	1.0000	1.0000	0.9142	0.9996	1.0000	1.0000	1.0000
5	0.9693	1.0000	0.8759	1.0000	.	1.0000	1.0000	0.9865	1.0000	1.0000	0.9710	1.0000	1.0000	1.0000	1.0000
6	0.9895	1.0000	0.9660	1.0000	1.0000	.	1.0000	0.9525	1.0000	1.0000	0.9746	1.0000	1.0000	1.0000	1.0000
7	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	0.9999	1.0000	1.0000	1.0000	1.0000
8	1.0000	0.9657	1.0000	0.8922	0.9865	0.9925	1.0000	.	0.9978	0.9972	1.0000	1.0000	0.9991	0.9447	0.8485
9	0.9981	1.0000	0.9964	1.0000	1.0000	1.0000	0.9978	0.9972	.	1.0000	0.9817	1.0000	1.0000	1.0000	1.0000
10	0.9964	1.0000	0.9874	1.0000	1.0000	1.0000	1.0000	0.9972	1.0000	.	0.9833	1.0000	1.0000	1.0000	1.0000
11	0.9999	0.9377	0.9998	0.9142	0.9710	0.9746	0.9999	1.0000	0.9817	0.9833	.	0.9992	0.9865	0.9010	0.9014
12	1.0000	0.9999	1.0000	0.9996	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9992	.	1.0000	0.9991	0.9993
13	0.9995	1.0000	0.9993	1.0000	1.0000	1.0000	1.0000	0.9991	1.0000	1.0000	0.9865	1.0000	.	1.0000	1.0000
14	0.9382	1.0000	0.9067	1.0000	1.0000	1.0000	1.0000	0.9447	1.0000	1.0000	0.9010	0.9991	1.0000	.	1.0000
15	0.6674	1.0000	0.2935	1.0000	1.0000	1.0000	1.0000	0.8485	1.0000	1.0000	0.9014	0.9993	1.0000	1.0000	.

Comparison of accident rates by ramp configuration, traffic volume as covariate, and district= predictor

Class Level Information
 Class Levels Values
 DISTRICT 12 1 2 3 4 5 6 7 8 9 10 11 12
 DESCR 15 buttonhook diamond3 diamond4 dir/semi3 dir/semi4 direct loopleft3 loopleft4 loopnoleft3 loopnoleft4 restarea scissors slip3 slip4 split
 Number of observations in by group = 5259
 NOTE: All dependent variable are consistent with respect to the presence or absence of missing values. However only 5257 observations can be used in this analysis.

Dependent Variable: **FIRATE**

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	26	165.32157264	6.35852202	6.29	0.0001
Error	5230	5290.10007615	1.01152964		
Corrected Total	5256	5455.62160879			
	R-Square	C.V.	Root MSE	FIRATE Mean	
	0.030303	338.5098	1.00574830	6.29711052	
Source	DF	Type I SS	Mean Square	F Value	Pr > F
DISTRICT	11	100.11934546	9.10175868	9.00	0.0001
RVOL	1	32.89377267	32.89377267	12.52	0.0001
DESCR	14	32.30845451	2.30774675	2.18	0.0042
Source	DF	Type III SS	Mean Square	F Value	Pr > F
DISTRICT	11	85.98862751	7.81714796	7.73	0.0001
RVOL	1	22.14391661	22.14391661	21.89	0.0001
DESCR	14	32.30845451	2.30774675	2.28	0.0042

Dependent Variable: **TRATE**

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	26	903.78661289	34.76102357	2.46	0.0001
Error	5230	73778.18724303	14.10672796		
Corrected Total	5256	74681.97385592			
	R-Square	C.V.	Root MSE	TRATE Mean	
	0.012102	461.2413	3.75589243	0.81436093	
Source	DF	Type I SS	Mean Square	F Value	Pr > F
DISTRICT	11	430.19509826	39.10864510	2.77	0.0014
RVOL	1	299.23450332	299.23450332	21.21	0.0001
DESCR	14	174.35701131	12.45407224	0.88	0.5775
Source	DF	Type III SS	Mean Square	F Value	Pr > F
DISTRICT	11	377.27849756	34.29804523	2.43	0.0051
RVOL	1	191.44764261	191.44764261	13.57	0.0002
DESCR	14	174.35701131	12.45407224	0.88	0.5775

Dependent Variable: **WRATE**

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	26	20.68595232	0.79561355	2.57	0.0001
Error	5230	1616.12801314	0.30984933		
Corrected Total	5256	1637.01396546			
	R-Square	C.V.	Root MSE	WRATE Mean	
	0.012436	413.8275	0.55592206	0.13433665	
Source	DF	Type I SS	Mean Square	F Value	Pr > F
DISTRICT	11	7.83606017	0.71236911	2.31	0.0082
RVOL	1	3.37069613	3.37069613	10.91	0.0010
DESCR	14	9.47919602	0.67708543	2.19	0.0063
Source	DF	Type III SS	Mean Square	F Value	Pr > F
DISTRICT	11	7.68404923	0.69854993	2.26	0.0097
RVOL	1	1.95356511	1.95356511	6.32	0.0120
DESCR	14	9.47919602	0.67708543	2.19	0.0063

Dependent Variable: **DRATE**

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	26	99.10808887	3.81954188	1.92	0.0034
Error	5230	10427.78232444	1.99383983		
Corrected Total	5256	10527.09041331			
	R-Square	C.V.	Root MSE	DRATE Mean	
	0.009434	576.2335	1.41203393	0.24504542	
Source	DF	Type I SS	Mean Square	F Value	Pr > F
DISTRICT	11	58.53748459	5.32158951	2.67	0.0020
RVOL	1	22.57745418	22.57745418	11.32	0.0008
DESCR	14	18.19315009	1.29951072	0.65	0.8228
Source	DF	Type III SS	Mean Square	F Value	Pr > F
DISTRICT	11	51.61639499	4.69239954	2.35	0.0068
RVOL	1	14.58346620	14.58346620	7.31	0.0069
DESCR	14	18.19315009	1.29951072	0.65	0.8228

comparison of accident rates by ramp configuration, traffic volume as covariate, and district= predictor

Least Squares Means

Adjustment for multiple comparisons: Tukey-Kramer

Dependent Variable: **FIRATE**

DESCR	FIRATE LSMEAN	LSMEAN Number
buttonhook	0.38907782	1
diamond3	0.32691993	2
diamond4	0.54638588	3
dir/semi3	0.42581529	4
dir/semi4	0.45081843	5
direct	0.52004433	6
loopleft3	0.46790138	7
loopleft4	0.56589073	8
loopnoleft3	0.38117742	9
loopnoleft4	0.45690042	10
restarea	0.14467324	11
scissors	0.39647272	12
slip3	0.48581635	13
slip4	0.71471026	14
split	0.38218968	15

Dependent Variable: **FIRATE**

i/j	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
1	.	1.0000	0.1671	1.0000	0.9996	0.9832	1.0000	0.6196	1.0000	0.9992	0.9822	1.0000	1.0000	0.3487	1.0000
2	1.0000	.	0.6043	0.9999	0.9968	0.9592	1.0000	0.7163	1.0000	0.9955	0.9997	1.0000	0.9996	0.3555	1.0000
3	0.1671	0.6043	.	0.8693	0.7356	1.0000	1.0000	1.0000	0.8318	0.8884	0.4683	0.9291	1.0000	0.9837	0.4047
4	1.0000	0.9999	0.8693	.	1.0000	0.9997	1.0000	0.9554	1.0000	1.0000	0.9527	1.0000	1.0000	0.6290	1.0000
5	0.9996	0.9968	0.7356	1.0000	.	1.0000	1.0000	0.9643	1.0000	1.0000	0.8794	1.0000	1.0000	0.6739	0.9998
6	0.9832	0.9592	1.0000	0.9997	1.0000	.	1.0000	1.0000	0.9962	1.0000	0.7401	0.9991	1.0000	0.9856	0.9785
7	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	0.9998	1.0000	1.0000	1.0000	1.0000
8	0.6196	0.7163	1.0000	0.9554	0.9643	1.0000	1.0000	.	0.9000	0.9829	0.4871	0.9551	1.0000	0.9984	0.7219
9	1.0000	1.0000	0.8318	1.0000	1.0000	0.9962	1.0000	0.9000	.	1.0000	0.9936	1.0000	1.0000	0.5318	1.0000
10	0.9992	0.9955	0.8884	1.0000	1.0000	1.0000	0.9829	1.0000	1.0000	.	0.8674	1.0000	1.0000	0.7268	0.9996
11	0.9822	0.9997	0.4683	0.9527	0.8794	0.7401	0.9998	0.4871	0.9936	0.8674	.	0.9888	0.9430	0.2069	0.9896
12	1.0000	1.0000	0.9291	1.0000	1.0000	0.9991	1.0000	0.9551	1.0000	1.0000	0.9888	.	1.0000	0.6332	1.0000
13	1.0000	0.9996	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9430	1.0000	.	0.9907	1.0000
14	0.3487	0.3555	0.9837	0.6290	0.6739	0.9856	1.0000	0.9984	0.5318	0.7268	0.2069	0.6332	0.9907	.	0.3668
15	1.0000	1.0000	0.4047	1.0000	0.9998	0.9785	1.0000	0.7219	1.0000	0.9996	0.9896	1.0000	1.0000	0.3668	.

Dependent Variable: **TRATE**

DESCR	TRATE LSMEAN	LSMEAN Number
buttonhook	0.81705397	1
diamond3	0.47346185	2
diamond4	1.10762758	3
dir/semi3	0.66347515	4
dir/semi4	0.71276531	5
direct	0.92782892	6
loopleft3	0.76173667	7
loopleft4	0.93686130	8
loopnoleft3	0.79830398	9
loopnoleft4	0.80708579	10
restarea	0.71435237	11
scissors	0.65261127	12
slip3	1.02260976	13
slip4	0.93772796	14
split	0.64860854	15

Dependent Variable: **TRATE**

i/j	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
1	.	0.9999	0.9806	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
2	0.9999	.	0.9128	1.0000	1.0000	0.9996	1.0000	0.9989	1.0000	0.9999	1.0000	1.0000	0.9998	1.0000	1.0000
3	0.9806	0.9128	.	0.8807	0.5729	1.0000	1.0000	1.0000	0.9997	0.9495	1.0000	0.9880	1.0000	1.0000	0.8490
4	1.0000	1.0000	0.8807	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
5	1.0000	1.0000	0.5729	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
6	1.0000	0.9996	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
7	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
8	1.0000	0.9989	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	0.9999
9	1.0000	1.0000	0.9997	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
10	1.0000	0.9999	0.9495	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000	1.0000
11	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000	1.0000
12	1.0000	1.0000	0.9880	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000	1.0000
13	1.0000	0.9998	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000	1.0000
14	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.	1.0000
15	1.0000	1.0000	0.8490	1.0000	1.0000	1.0000	1.0000	0.9999	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	.

Dependent Variable: **WRATE**

DESCR	WRATE LSMEAN	LSMEAN Number
buttonhook	0.07569713	1
diamond3	0.04107740	2
diamond4	0.11912743	3
dir/semi3	0.09499567	4
dir/semi4	0.10965142	5
direct	0.15273443	6
loopleft3	-0.30504391	7
loopleft4	0.14239196	8
loopnoleft3	0.20869104	9
loopnoleft4	0.15178763	10
restarea	-0.03357429	11
scissors	0.08508472	12
slip3	0.04168596	13
slip4	0.21851993	14
split	0.06733281	15

Dependent Variable: **WRATE**

i/j	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
1		1.0000	0.9789	1.0000	0.9996	0.9708	1.0000	0.9698	0.4079	0.6862	0.9978	1.0000	1.0000	0.0299	1.0000
2	1.0000		0.9809	0.9999	0.9968	0.9423	1.0000	0.9525	0.4900	0.8345	1.0000	1.0000	1.0000	0.0506	1.0000
3	0.9789	0.9809		1.0000	1.0000	1.0000	1.0000	1.0000	0.8530	0.9972	0.8355	1.0000	0.9989	0.1237	0.9820
4	1.0000	0.9999	1.0000		1.0000	0.9990	1.0000	0.9997	0.7777	0.9879	0.9913	1.0000	1.0000	0.1036	1.0000
5	0.9996	0.9968	1.0000	1.0000		0.9999	1.0000	1.0000	0.8074	0.9911	0.8665	1.0000	0.9998	0.1080	0.9991
6	0.9708	0.9423	1.0000	0.9990	0.9999		0.9999	1.0000	0.9999	1.0000	0.8638	0.9992	0.9886	0.6723	0.9438
7	1.0000	1.0000	1.0000	1.0000	1.0000	0.9999		0.9999	0.9966	0.9999	1.0000	1.0000	1.0000	0.9976	1.0000
8	0.9698	0.9525	1.0000	0.9997	1.0000	1.0000	0.9999		0.9980	1.0000	0.8810	0.9997	0.9937	0.4804	0.9656
9	0.4079	0.4900	0.8530	0.7777	0.8074	0.9999	0.9966	0.9980		0.9987	0.4991	0.8495	0.7822	0.9860	0.4125
10	0.6862	0.8345	0.9972	0.9879	0.9911	1.0000	0.9999	1.0000	0.9987		0.7927	0.9946	0.9749	0.4613	0.7497
11	0.9978	1.0000	0.9355	0.9913	0.9665	0.8638	1.0000	0.8810	0.4991	0.7927		0.9978	1.0000	0.0854	0.9993
12	1.0000	1.0000	1.0000	1.0000	1.0000	0.9992	1.0000	0.9997	0.8495	0.9946	0.9978		1.0000	0.1628	1.0000
13	1.0000	1.0000	0.9989	1.0000	0.9998	0.9886	1.0000	0.9937	0.7822	0.9749	1.0000	1.0000		0.1662	1.0000
14	0.0299	0.0506	0.1237	0.1036	0.1080	0.6723	0.8776	0.4804	0.9860	0.4613	0.0854	0.1628	0.1662		0.0290
15	1.0000	1.0000	0.9820	1.0000	0.9991	0.9438	1.0000	0.9656	0.4125	0.7497	0.9993	1.0000	1.0000	0.0290	

Dependent Variable: **DRATE**

DESCR	DRATE LSMEAN	LSMEAN Number
buttonhook	0.14420932	1
diamond3	0.08817077	2
diamond4	0.25074289	3
dir/semi3	0.14731175	4
dir/semi4	0.14119938	5
direct	0.20078626	6
loopleft3	0.05629188	7
loopleft4	0.20392013	8
loopnoleft3	0.19759493	9
loopnoleft4	0.15065034	10
restarea	0.04812046	11
scissors	0.10548876	12
slip3	0.10646089	13
slip4	0.09822058	14
split	0.12196675	15

Dependent Variable: **DRATE**

i/j	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
1		1.0000	0.9659	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
2	1.0000		0.9950	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
3	0.9659	0.9950		0.9958	0.8677	1.0000	1.0000	1.0000	1.0000	0.9591	0.9998	0.9954	0.9999	0.9997	0.9711
4	1.0000	1.0000	0.9958		1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
5	1.0000	1.0000	0.8677	1.0000		1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
6	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
7	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
8	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
9	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
10	1.0000	1.0000	0.9591	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000	1.0000	1.0000	1.0000
11	1.0000	1.0000	0.9998	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000	1.0000	1.0000
12	1.0000	1.0000	0.9954	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000	1.0000
13	1.0000	1.0000	0.9999	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000
14	1.0000	1.0000	0.9997	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000
15	1.0000	1.0000	0.9711	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	

Appendix D Ramps with Relatively High Accident Rates

Appendix D

Ramps with a relatively high accident rate

Dist	Route	County	Post Mile	Rural	Description	On/Off
1	101	DN	3.64	r	buttonhook	off
1	101	DN	4.76	r	diamond3	on
1	101	HUM	85.129	u	dir/semi4	off
1	101	HUM	27.973	r	loopleft3	off
1	101	HUM	64.404	r	loopleft3	off
1	101	HUM	61.384	u	scissors	off
1	101	HUM	61.384	u	scissors	off
1	101	HUM	70.851	r	twowayseg4	
1	101	MEN	82.43	r	restarea	off
1	101	MEN	82.43	r	restarea	off
2	5	SHA	66	r	dir/semi4	off
2	5	SHA	49.375	r	dir/semi4	on
2	5	SIS	19.61	r	diamond3	off
2	5	SIS	25.345	r	restarea	off
2	5	SIS	25.345	r	restarea	off
3	5	SAC	12.188	r	loopnoleft4	on
3	5	YOL	0.67	r	buttonhook	on
3	5	YOL	8.191	u	loopleft3	off
3	50	SAC	15.811	u	loopnoleft4	on
3	51	SAC	6.181	u	scissors	off
3	51	SAC	6.181	u	scissors	off
3	80	NEV	8.86	r	diamond4	off
3	80	NEV	29.39	r	diamond4	off
3	80	NEV	59.83	r	loopnoleft3	off
3	80	PLA	28.45	r	buttonhook	on
3	80	PLA	37.93	r	buttonhook	on
3	80	PLA	43.27	r	diamond4	off
3	80	PLA	33.08	r	loopleft4	off
3	80	PLA	56.755	r	loopnoleft4	off
3	80	YOL	11.04	u	diamond3	off
3	80	YOL	11.219	u	loopleft3	on
3	99	SAC	5.925	r	loopleft4	off
3	99	SAC	19.71	u	loopnoleft4	on
3	99	SUT	5.534	r	twowayseg4	
3	113	YOL	10.079	u	loopleft3	on
3	160	SAC	44.983	u	dir/semi4	on
3	505	YOL	10.86	r	dir/semi3	on
3	505	YOL	0.67	r	dir/semi4	on
3	80	PLA	3.611	u	dir/semi4	on

Appendix D Ramps with Relatively High Accident Rtaes

Dist	Route	County	Post Mile	Rural	Description	On/Off
3	99	SAC	17.913	u	dir/semi4	on
4	1	SM	47.604	u	dir/semi3	off
4	1	SM	46.884	u	dir/semi3	on
4	1	SM	46.811	u	direct	off
4	4	CC	4.627	u	scissors	off
4	4	CC	4.627	u	scissors	off
4	12	SON	14.255	u	dir/semi4	on
4	13	ALA	5.48	u	buttonhook	on
4	80	ALA	2.17	u	direct	on
4	80	ALA	5.81	u	scissors	off
4	80	ALA	5.81	u	scissors	off
4	80	CC	12.535	u	diamond4	on
4	80	CC	9.99	u	dir/semi3	on
4	85	SCL	7.96	u	buttonhook	off
4	85	SCL	4.031	u	dir/semi4	on
4	87	SCL	1.53	u	buttonhook	on
4	87	SCL	4.994	u	direct	off
4	87	SCL	6.097	u	slip3	on
4	87	SCL	6.097	u	slip3	on
4	92	SM	11.321	u	dir/semi3	off
4	92	SM	7.361	u	loopnoleft3	off
4	101	MRN	18.725	u	direct	on
4	101	MRN	0.312	u	scissors	on
4	101	MRN	0.312	u	scissors	on
4	101	MRN	19.036	u	split	on
4	101	MRN	19.257	u	split	on
4	101	SCL	30.15	u	buttonhook	on
4	101	SCL	50.401	u	dir/semi4	off
4	101	SCL	33.171	u	loopnoleft4	off
4	101	SCL	50.271	u	loopnoleft4	off
4	101	SCL	36.078	u	slip3	on
4	101	SCL	36.078	u	slip3	on
4	101	SF	2.741	u	slip4	off
4	101	SM	25.844	u	loopnoleft4	on
4	101	SM	23.261	u	scissors	off
4	101	SM	23.261	u	scissors	off
4	101	SM	11.354	u	split	on
4	101	SON	3.747	u	loopleft3	off
4	101	SON	18.82	u	scissors	on
4	101	SON	18.82	u	scissors	on
4	101	SON	26.476	u	split	on
4	280	SM	8.416	r	restarea	off

Appendix D Ramps with Relatively High Accident Rtaes

Dist	Route	County	Post Mile	Rural	Description	On/Off
4	280	SM	7.562	r	restarea	on
4	280	SM	8.871	r	restarea	on
4	280	SM	8.416	r	restarea	off
4	280	SM	7.562	r	restarea	on
4	280	SM	8.871	r	restarea	on
4	580	ALA	28.36	u	loopleft4	on
4	580	ALA	45.581	u	slip3	off
4	580	ALA	45.581	u	slip3	off
4	580	CC	4.898	u	slip3	off
4	580	CC	4.898	u	slip3	off
4	580	MRN	4.622	u	slip3	off
4	580	MRN	4.622	u	slip3	off
4	680	ALA	7.481	u	loopleft4	off
4	680	ALA	8.684	r	restarea	off
4	680	ALA	8.861	r	restarea	on
4	680	ALA	8.684	r	restarea	off
4	680	ALA	8.861	r	restarea	on
4	680	CC	10.211	u	dir/semi4	off
4	680	CC	19.702	u	dir/semi4	on
4	680	CC	16.251	u	restarea	off
4	680	CC	16.704	u	restarea	on
4	680	CC	16.251	u	restarea	off
4	680	CC	16.704	u	restarea	on
4	680	SCL	3.713	u	dir/semi4	off
4	680	SCL	3.547	u	split	off
4	880	ALA	27.882	u	direct	on
4	880	ALA	15.516	u	loopnoleft4	off
4	880	ALA	27.541	u	scissors	off
4	880	ALA	27.541	u	scissors	off
4	880	ALA	32.089	u	split	on
4	880	SCL	10.484	u	loopleft4	off
4	880	SCL	8.613	u	loopnoleft3	off
4	85	SCL	8.234	u	diamond4	on
5	1	SLO	29.84	u	scissors	on
5	1	SLO	29.84	u	scissors	on
5	68	MON	4.22	u	split	off
5	101	MON	63.166	r	diamond4	on
5	101	MON	61.49	r	loopleft3	off
5	101	MON	2.09	r	loopleft4	off
5	101	MON	60.45	r	loopleft4	off
5	101	MON	1.94	r	scissors	on
5	101	MON	1.94	r	scissors	on

Appendix D Ramps with Relatively High Accident Rates

Dist	Route	County	Post Mile	Rural	Description	On/Off
5	101	MON	95.484	u	twowayseg4	
5	101	SB	29.74	r	buttonhook	off
5	101	SB	78.83	r	buttonhook	off
5	101	SB	11.74	u	dir/semi3	off
5	101	SB	34.2	r	dir/semi4	off
5	101	SBT	4.70	r	diamond3	on
5	101	SBT	2.74	r	dir/semi3	on
5	101	SBT	2.88	r	loopnoleft3	off
5	101	SLO	54.26	u	diamond3	off
6	5	FRE	0.41	r	diamond3	on
6	5	FRE	30.11	r	diamond3	on
6	5	FRE	59.86	r	diamond4	off
6	5	KER	4.749	r	buttonhook	on
6	5	KER	19.83	r	diamond3	off
6	5	KER	56.525	r	diamond4	on
6	5	KER	10.281	r	loopnoleft4	off
6	5	KER	0.691	r	restarea	off
6	5	KER	1.512	r	scissors	on
6	5	KER	0.691	r	restarea	off
6	5	KER	1.512	r	scissors	on
6	65	TUL	18.041	u	loognoleft3	off
6	99	FRE	25.81	u	buttonhook	off
6	99	FRE	16.698	u	diamond4	on
6	99	FRE	26.419	u	dir/semi4	on
6	99	FRE	10.777	r	scissors	on
6	99	FRE	10.777	r	scissors	on
6	99	FRE	20.908	u	slip4	off
6	99	KER	5.10	r	buttonhook	off
6	99	KER	57.453	u	buttonhook	off
6	99	KER	10.531	r	direct	on
6	99	KER	44.319	r	loopleft3	off
6	99	KER	2.528	r	loopleft3	on
6	99	KER	5.414	r	loopleft4	on
6	99	KER	26.874	u	twowayseg4	
6	99	MAD	22.88	r	diamond3	off
6	99	MAD	12.722	u	loopleft4	off
6	99	TUL	18.396	r	loopleft3	on
6	99	TUL	9.105	r	scissors	off
6	99	TUL	9.105	r	scissors	off
6	198	KIN	8.679	r	dir/semi3	off
6	198	KIN	4.981	r	loopleft4	on
6	198	TUL	9.51	u	slip3	on

Appendix D Ramps with Relatively High Accident Rtaes

Dist	Route	County	Post Mile	Rural	Description	On/Off
6	198	TUL	9.51	u	slip3	on
6	198	TUL	10.187	u	slip4	off
7	1	LA	34.829	u	slip3	off
7	1	LA	34.829	u	slip3	off
7	1	VEN	13.611	r	scissors	off
7	1	VEN	16.001	u	scissors	on
7	1	VEN	13.611	r	scissors	off
7	1	VEN	16.001	u	scissors	on
7	2	LA	14.989	u	slip3	off
7	2	LA	14.989	u	slip3	off
7	2	LA	14.851	u	split	off
7	5	LA	20.452	u	dir/semi3	off
7	5	LA	18.208	u	direct	on
7	5	LA	36.155	u	loopleft3	on
7	5	LA	55.531	u	loopleft3	on
7	5	LA	52.46	u	loopleft4	on
7	5	LA	25.852	u	loopnoleft3	on
7	5	LA	15.165	u	scissors	off
7	5	LA	20.802	u	slip3	on
7	5	LA	15.165	u	scissors	off
7	5	LA	20.802	u	slip3	on
7	5	LA	44.175	u	slip4	off
7	10	LA	31.21	u	loognoleft3	on
7	10	LA	23.288	u	loopnoleft4	off
7	10	LA	23.376	u	loopnoleft4	off
7	10	LA	25.296	u	loopnoleft4	off
7	10	LA	2.347	u	slip3	off
7	10	LA	42.5	u	slip3	on
7	10	LA	21.636	u	slip3	on
7	10	LA	2.347	u	slip3	off
7	10	LA	42.5	u	slip3	on
7	10	LA	21.636	u	slip3	on
7	10	LA	14.985	u	split	off
7	10	LA	0.09	u	split	on
7	14	LA	54.38	r	buttonhook	off
7	14	LA	51.75	r	buttonhook	on
7	14	LA	28.50	u	diamond3	off
7	14	LA	73.84	r	diamond3	on
7	14	LA	59.97	u	dir/semi3	on
7	14	LA	73.965	r	loopnoleft3	on
7	14	LA	56.783	u	restarea	of f
7	14	LA	56.783	u	restarea	off

Appendix D Ramps with Relatively High Accident Rates

Dist	Route	County	Post Mile	Rural	Description	On/Off
7	23	VEN	7.135	u	loopnoleft4	on
7	33	VEN	0.453	u	direct	off
7	60	LA	2.974	u	split	off
7	60	LA	0.511	u	twowayseg4	
7	90	LA	1.589	u	dir/semi4	on
7	101	LA	5.46	u	diamond3	on
7	101	LA	0.606	u	split	on
7	103	LA	1.429	u	dir/semi3	on
7	103	LA	1.488	u	loopnoleft3	off
7	105	LA	13.018	u	direct	on
7	105	LA	13.999	u	slip3	on
7	105	LA	13.999	u	slip3	on
7	105	LA	14.901	u	slip4	on
7	105	LA	14	u	split	on
7	110	LA	14.093	u	direct	off
7	110	LA	29.989	u	direct	off
7	110	LA	24.727	u	scissors	off
7	110	LA	24.727	u	scissors	off
7	110	LA	18.829	u	slip4	off
7	110	LA	14.31	u	slip4	on
7	110	LA	18.647	u	slip4	on
7	110	LA	25.418	u	split	on
7	110	LA	26.176	u	split	on
7	118	LA	10.21	u	diamond3	off
7	118	LA	12.375	u	loopleft4	on
7	126	VEN	1.564	u	loopnoleft4	off
7	126	VEN	5.143	u	loopnoleft4	off
7	134	LA	11.425	u	loopleft4	off
7	170	LA	15.03	u	buttonhook	on
7	210	LA	36.206	u	dir/semi3	on
7	210	LA	0.4	u	direct	off
7	405	LA	20.29	u	buttonhook	off
7	405	LA	20.34	u	buttonhook	on
7	405	LA	21.19	u	buttonhook	on
7	405	LA	20.354	u	dir/semi3	on
7	405	LA	17.625	u	loopleft3	on
7	405	LA	46.286	u	loopnoleft3	on
7	405	LA	20.353	u	loopnoleft4	on
7	405	LA	22.201	u	loopnoleft4	on
7	405	LA	32.134	u	scissors	off
7	405	LA	32.134	u	scissors	off
7	605	LA	8.58	u	diamond3	off

Appendix D Ramps with Relatively High Accident Rtaes

Dist	Route	County	Post Mile	Rural	Description	On/Off
7	605	LA	19.296	u	direct	on
7	605	LA	9.686	u	loopleft3	on
7	605	LA	22.366	u	slip4	on
7	605	LA	14.234	u	split	on
7	710	LA	23.50	u	buttonhook	on
7	710	LA	12.156	u	dir/semi4	off
7	710	LA	21.808	u	dir/semi4	off
7	710	LA	15.899	u	direct	on
8	10	RIV	12.69	u	diamond3	off
8	10	RIV	26.07	r	restarea	off
8	10	RIV	15.408	u	restarea	on
8	10	RIV	26.07	r	restarea	off
8	10	RIV	15.408	u	restarea	on
8	10	SBD	25.62	u	buttonhook	off
8	10	SBD	11.465	u	split	off
8	10	SBD	29.687	u	split	off
8	15	SBD	55.795	r	diamond4	off
8	15	SBD	76.25	r	diamond4	off
8	15	SBD	55.815	r	diamond4	on
8	15	SBD	31.256	r	direct	off
8	15	SBD	41.496	u	loopleft4	off
8	15	SBD	87.157	r	restarea	of f
8	15	SBD	60.996	r	restarea	on
8	15	SBD	70.72	r	restarea	on
8	15	SBD	87.157	r	restarea	off
8	15	SBD	60.996	r	restarea	on
8	15	SBD	70.72	r	restarea	on
8	30	SBD	26.52	u	diamond4	off
8	30	SBD	27.063	u	diamond4	off
8	30	SBD	26.644	u	diamond4	on
8	40	SBD	99.469	r	diamond4	on
8	40	SBD	38.812	u	direct	off
8	40	SBD	0.769	u	loopleft3	on
8	40	SBD	48.988	r	restarea	off
8	40	SBD	5.641	r	restarea	on
8	40	SBD	48.988	r	restarea	off
8	40	SBD	5.641	r	restarea	on
8	60	RIV	30.41	u	dir/semi3	on
8	60	RIV	20.368	u	loopleft4	on
8	71	SBD	0.198	u	dir/semi4	off
8	91	RIV	2.573	u	direct	on
8	215	RIV	26.46	u	diamond3	off

Appendix D Ramps with Relatively High Accident Rtaes

Dist	Route	County	Post Mile	Rural	Description	On/Off
8	215	RIV	23.545	u	loopleft3	off
8	215	RIV	23.716	u	loopleft4	on
8	215	RiV	43.342	u	loopnoleft3	on
8	215	RIV	36.474	u	loopnoleft4	on
8	215	SBD	0.055	u	scissors	on
8	215	SBD	0.055	u	scissors	on
8	30	SBD	29.216	u	diamond4	off
9	14	KER	0.17	u	diamond4	on
9	58	KER	42.819	r	loopleft4	on
10	4	SJ	17.969	u	slip4	on
10	5	MER	17.38	r	dir/semi3	off
10	5	MER	17.57	r	loopnoleft3	off
10	5	MER	17.488	r	loopnoleft3	on
10	5	SJ	23.49	u	diamond4	off
10	5	SJ	3.175	, r	dir/semi3	off
10	5	SJ	39.36	r	dir/semi3	on
10	5	SJ	26.454	u	split	on
10	12	SOL	4.277	u	direct	off
10	80	SOL	2.89	u	buttonhook	off
10	80	SOL	5.89	u	diamond3	off
10	80	SOL	38.18	r	dir/semi3	off
10	80	SOL	43.483	u	dir/semi3	off
10	80	SOL	5.442	u	dir/semi4	off
10	80	SOL	0.954	u	direct	off
10	80	SOL	12.981	u	loopnoleft3	on
10	80	SOL	1.219	u	scissors	off
10	80	SOL	1.219	u	scissors	off
10	99	MER	15.93	u	diamond3	off
10	99	MER	15.95	u	diamond3	on
10	99	SJ	22.07	u	buttonhook	off
10	99	SJ	6.84	u	diamond3	on
10	99	SJ	11.655	r	diamond4	on
10	99	SJ	19.136	u	dir/semi4	on
10	99	SJ	18.443	u	direct	off
10	99	SJ	21.723	u	direct	on
10	99	SJ	27.514	u	loopleft4	on
10	99	SJ	16.684	u	split	off
10	99	STA	15.377	u	slip4	off
10	99	STA	15.454	u	slip4	on
10	99	STA	13.617	u	split	off
10	120	SJ	4.087	u	diamond4	off
10	120	SJ	3.031	u	diamond4	on

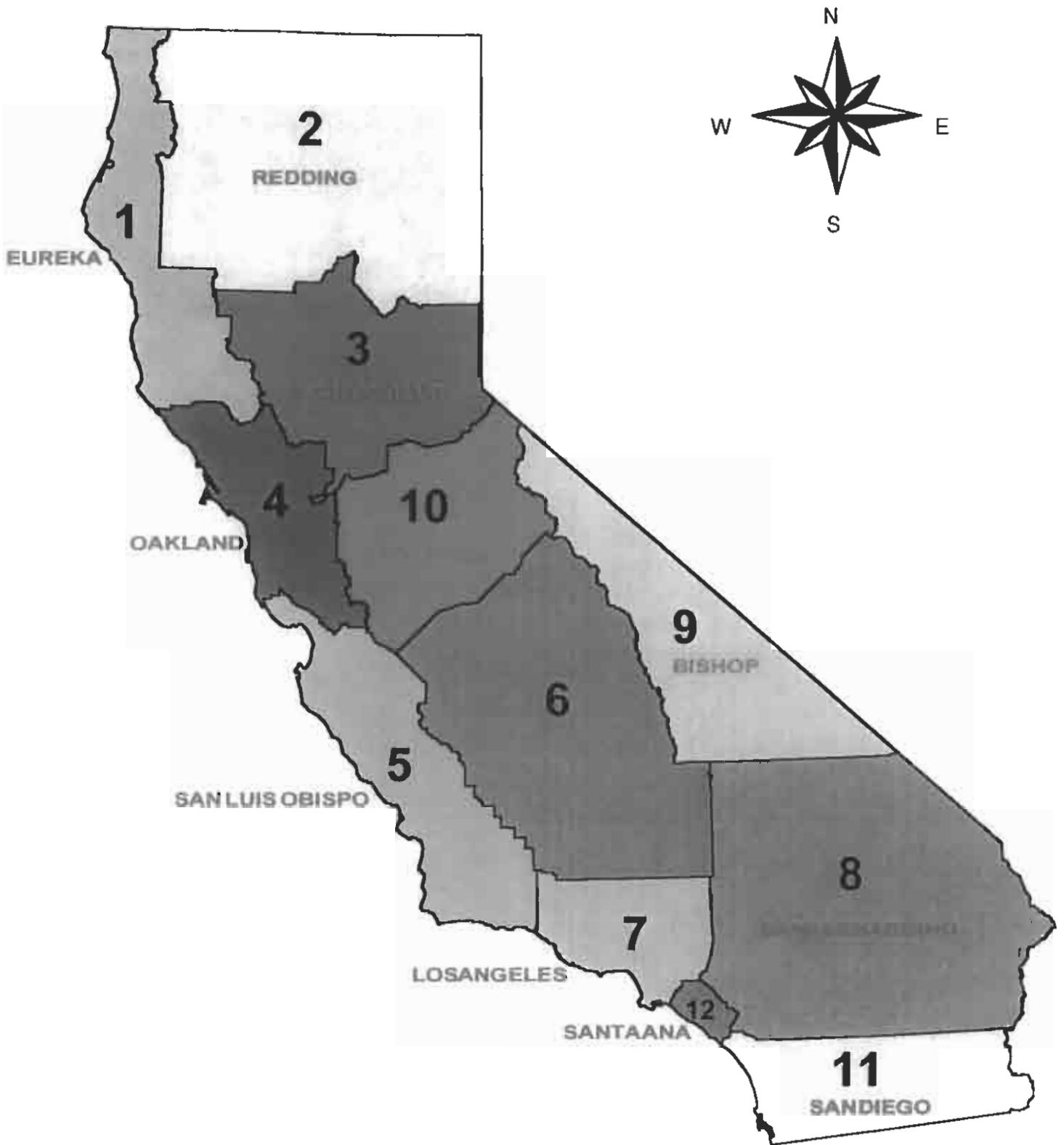
Appendix D Ramps with Relatively High Accident Rtaes

Dist	Route	County	Post Mile	Rural	Description	On/Off
10	505	SOL	10.35	r	buttonhook	off
10	505	SOL	10.43	r	buttonhook	on
10	505	SOL	10.383	r	loopleft4	off
11	5	SD	0.24	u	buttonhook	on
11	5	SD	3.355	u	dir/semi3	on
11	5	SD	11.13	u	loopleft4	off
11	5	SD	12.688	u	loopnoleft3	off
11	5	SD	51.252	u	loopnoleft3	off
11	5	SD	15.772	u	slip4	off
11	8	IMP	41.166	r	dir/semi3	on
11	8	IMP	3.73	r	restarea	off
11	8	IMP	3.73	r	restarea	off
11	8	SD	2.642	u	dir/semi3	off
11	8	SD	35.623	r	restarea	on
11	8	SD	10.69	u	scissors	off
11	8	SD	35.623	r	restarea	on
11	8	SD	10.69	u	scissors	off
11	8	SD	5.726	u	slip3	on
11	10	RIV	49.01	r	diamond3	on
11	10	RIV	59.06	u	diamond3	on
11	15	SD	6.205	u	loopnoleft3	on
11	15	SD	43.17	r	loopnoleft4	on
11	67	SD	4.856	u	loopnoleft4	on
11	94	SD	2.703	u	loopleft4	on
11	125	SD	15.208	u	direct	on
11	125	SD	15.47	u	loognoleft3	off
11	163	SD	1.52	u	direct	off
11	163	SD	2.699	u	direct	on
11	163	SD	0.753	u	twowayseg4	
11	209	SD	7.764	u	loognoleft3	off
11	805	SD	21.499	u	dir/semi3	off
11	805	SD	8.592	u	loopnoleft3	off
12	5	ORA	28.456	u	diamond4	off
12	5	ORA	23.772	u	diamond4	on
12	5	ORA	28.417	u	diamond4	on
12	5	ORA	28.224	u	loopleft4	off
12	55	ORA	2.512	u	slip3	off
12	55	ORA	3.506	u	slip3	off
12	55	ORA	2.512	u	slip3	off
12	55	ORA	3.506	u	slip3	off
12	55	ORA	3.091	u	slip3	on
12	55	ORA	3.589	u	slip3	on

Appendix D Ramps with Relatively High Accident Rtaes

Dist	Route	County	Post Mile	Rural	Description	On/Off
12	55	ORA	3.988	u	slip3	on
12	57	ORA	17.413	u	slip4	off
12	405	ORA	20.851	u	dir/semi4	off
12	405	ORA	3.04	u	dir/semi4	on
12	405	ORA	7.758	u	loopleft3	on
12	405	ORA	9.631	u	loopnoleft4	off

Appendix E



CALTRANS DISTRICT MAP

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Appendix F

Accident Analysis for Half Diamond Ramps

Analysis of the following half diamond ramp sample is included in this appendix at the request of Caltrans Office of Project Planning and Design (OPPD). Five years of accident data for the following 50 half diamond ramps was included in this analysis. The ramp identification designation number and corresponding location of ramps are shown in Table 20. All the ramps in the table are in urban or suburban area⁵. Since fatal accidents are very rare on ramps their random occurrence could not provide a reliable means of describing the safety of a ramp. Therefore, this analysis focused on *total* and *fatal + injury* accidents. Accident frequency, accident rate, and traffic volume for ramp cross street and ramp itself are plotted in figure 20 through 25. Consideration of either accident frequency or accident rate alone may lead to unrealistic assessment of how safe a ramp is.

Since the accident occurrence for off-ramps are significantly different than on-ramps, comparison of ramp accident frequency, accident rate and traffic volume are done separately. Figure 20 through 22 correspond to off-ramps and figure 23 through 25 correspond to the on-ramps. Each of the above plots are accompanied by descriptive statistics which provide the mean, standard deviation, maximum and the minimum for the ramps specified with ramp identification numbers.

Notice that in figure 21 and figure 24, total, and fatal + injury accident rates are compared to their corresponding *average* accident rates. The average values plotted are slightly different than the mean values in the descriptive statistics provided with each of above figures. The mean values in the descriptive statistics represent the average accident rate of the ramp sample specified in the plots

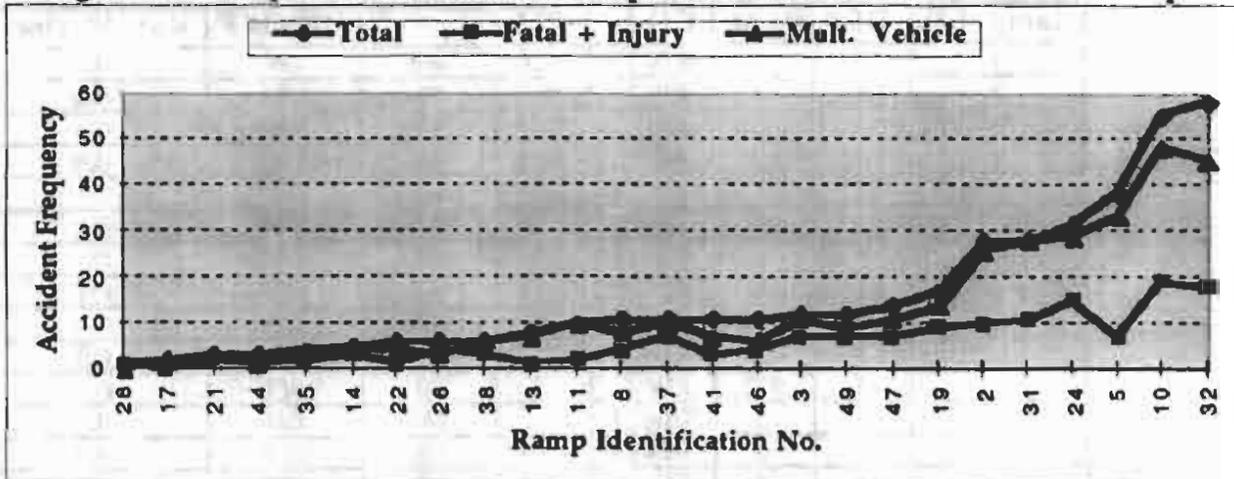
⁵ Suburban is defined as urbanized area (population over 50,000) which is entirely inside city limit.

(Ramp Identification No.). The average accident rate lines plotted in figure 19 through 25 however, are based on averaging much larger number of ramps in TASAS database. TASAS database provides the mean values for every single ramp based on averaging accident rates for the entire population of ramps on California state highways which has similar characteristics to the specific ramp.

Table 20 Half Diamond Ramp Location and Characteristics

Ramp I.D.	Dist	Route	CO.		PM		ON/OFF	Rural/Urban
1	8	91	RIV		10.6	WB	ON	(U)
2	8	91	RIV		10.6	EB	OFF	(U)
3	8	91	RIV		20.7	WB	OFF	(U)
4	8	91	RIV		20.7	EB	ON	(U)
5	8	60	RIV	R	1.51	EB	OFF	(S)
6	8	60	RIV	R	1.56	WB	ON	(S)
7	8	60	RIV	R	2.22	EB	ON	(S)
8	8	60	RIV	R	2.23	WB	OFF	(S)
9	8	215	RIV		43	NB	ON	(U)
10	8	215	RIV		43	NB	OFF	(U)
11	8	215	RIV		43	SB	OFF	(U)
12	2	5	SHA	R	4.42	NB	ON	(U)
13	2	5	SHA	R	4.45	SB	OFF	(U)
14	4	4	CC		26.8	EB	OFF	(U)
15	4	4	CC		26.8	WB	ON	(U)
16	8	10	RIV		8.42	EB	ON	(U)
17	8	10	RIV		8.44	WB	OFF	(U)
18	3	5	SAC		18.8	NB	ON	(U)
19	3	5	SAC		18.8	SB	OFF	(U)
20	8	10	SBD		33	WB	ON	(U)
21	8	10	SBD		33	EB	OFF	(U)
22	8	10	SBD		31.2	WB	OFF	(U)
23	8	10	SBD		31.2	EB	ON	(U)
24	8	10	SBD		31.7	EB	OFF	(U)
25	8	10	SBD		31.8	WB	ON	(U)
26	8	10	SBD		32.3	WB	OFF	(U)
27	8	10	SBD		32.3	EB	ON	(U)
28	8	10	SBD		34.4	WB	OFF	(S)
29	8	10	SBD		34.4	EB	ON	(S)
30	4	580	ALA	R	40.2	WB	ON	(U)
31	4	580	ALA	R	40.2	EB	OFF	(U)
32	4	580	ALA	R	40.8	EB	OFF	(U)
33	4	580	ALA	R	40.8	WB	ON	(U)
34	4	85	SCL	R	11.2	NB	ON	(U)
35	4	85	SCL	R	11.2	SB	OFF	(U)
36	4	680	CC	R	12.4	SB	ON	(U)
37	4	680	CC	R	12.5	NB	OFF	(U)
38	11	125	SD		14.6	NB	OFF	(U)
39	11	125	SD		14.6	SB	ON	(U)
40	7	5	LA		36.2	SB	ON	(U)
41	7	5	LA		36.4	SB	OFF	(U)
42	7	5	LA		36.6	NB	ON	(U)
43	7	210	LA	R	20.4	WB	ON	(U)
44	7	210	LA	R	20.4	EB	OFF	(U)
45	7	210	LA	R	21.1	EB	ON	(U)
46	7	210	LA	R	21.2	WB	OFF	(U)
47	7	101	LA		24.7	NB	OFF	(U)
48	7	101	LA		24.7	SB	ON	(U)
49	7	101	LA		16.7	NB	OFF	(U)
50	7	101	LA		16.8	SB	ON	(U)

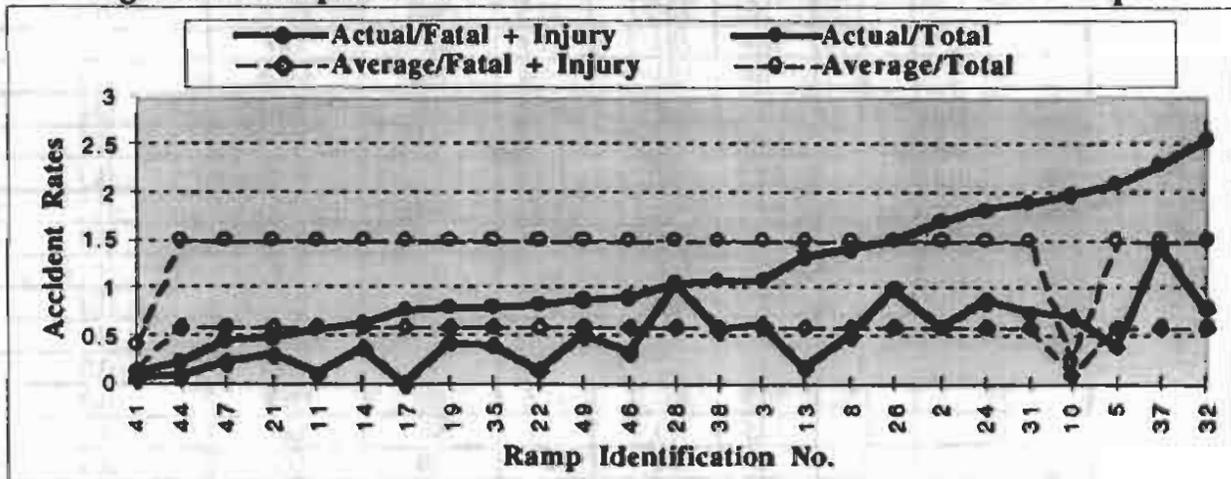
Figure 20 Comparison of Accident Frequencies for Half Diamond Off-Ramps



Descriptive Statistics

	Mean	Std. Dev.	Std. Error	Count	Minimum	Maximum	# Missing
Total Acc.	15.640	15.599	3.120	25	1.000	58.000	0
Fat+Inj. Acc.	5.920	5.291	1.058	25	0.000	19.000	0
Mult Veh Acc.	13.120	13.590	2.718	25	0.000	48.000	0

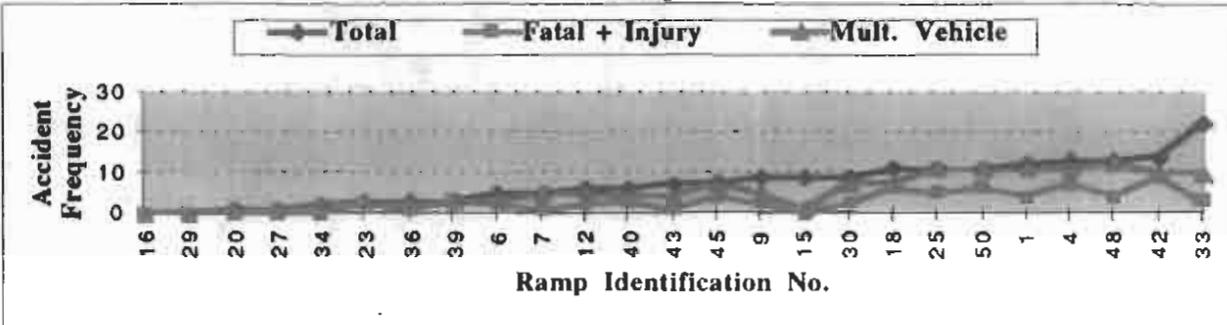
Figure 21 Comparison of Accident Rates for Half Diamond Off-Ramps



Descriptive Statistics

	Mean	Std. Dev.	Std. Error	Count	Minimum	Maximum	# Missing
Actual F+I Accident Rate	.496	.356	.071	25	0.000	1.460	0
Actual Total Accident Rate	1.171	.662	.132	25	.130	2.560	0
Average F+I Accident Rate	.553	.129	.026	25	.090	.590	0
Average Total Accident Rate	1.406	.326	.065	25	.250	1.500	0

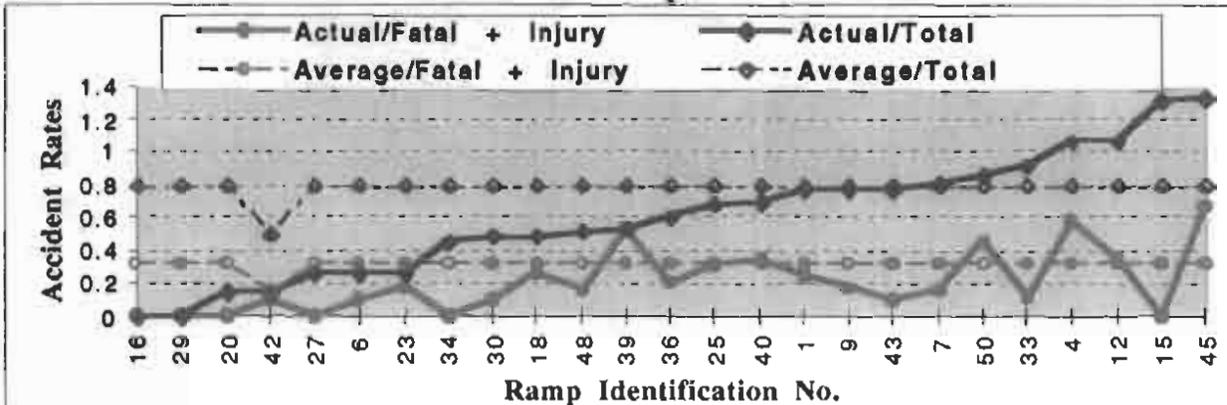
Figure 22 Comparison of Accident Frequencies for Half Diamond On-Ramps



Descriptive Statistics

	Mean	Std. Dev.	Std. Error	Count	Minimum	Maximum	# Missing
Total Accidents	7.360	5.330	1.066	25	0.000	22.000	0
Fatal Accidents	2.680	2.462	.492	25	0.000	9.000	0
Mult Veh Accidents	5.480	4.001	.800	25	0.000	12.000	0

Figure 23 Comparison of Accident Rates for Half Diamond On-Ramps



Descriptive Statistics

	Mean	Std. Dev.	Std. Error	Count	Minimum	Maximum	# Missing
Actual F+I Accident Rate	.207	.191	.038	25	0.000	.660	0
Actual Total Accident Rate	.607	.370	.074	25	0.000	1.330	0
Average F+I Accident Rate	.314	.032	.006	25	.160	.320	0
Average Total Accident Rate	.788	.060	.012	25	.500	.800	0

1. $\frac{1}{x^2} = x^{-2}$
 $\frac{d}{dx} x^{-2} = -2x^{-3} = -\frac{2}{x^3}$



x	y
-3	-2/27
-2	-1/4
-1	2
0	undefined
1	-2
2	-1/4
3	-2/27

2. $\frac{1}{x^3} = x^{-3}$
 $\frac{d}{dx} x^{-3} = -3x^{-4} = -\frac{3}{x^4}$



x	y
-3	-3/81 = -1/27
-2	-3/16
-1	-3
0	undefined
1	-3
2	-3/16
3	-1/27

Figure 24 Traffic Volume on Half Diamond Off-Ramps and Cross Street

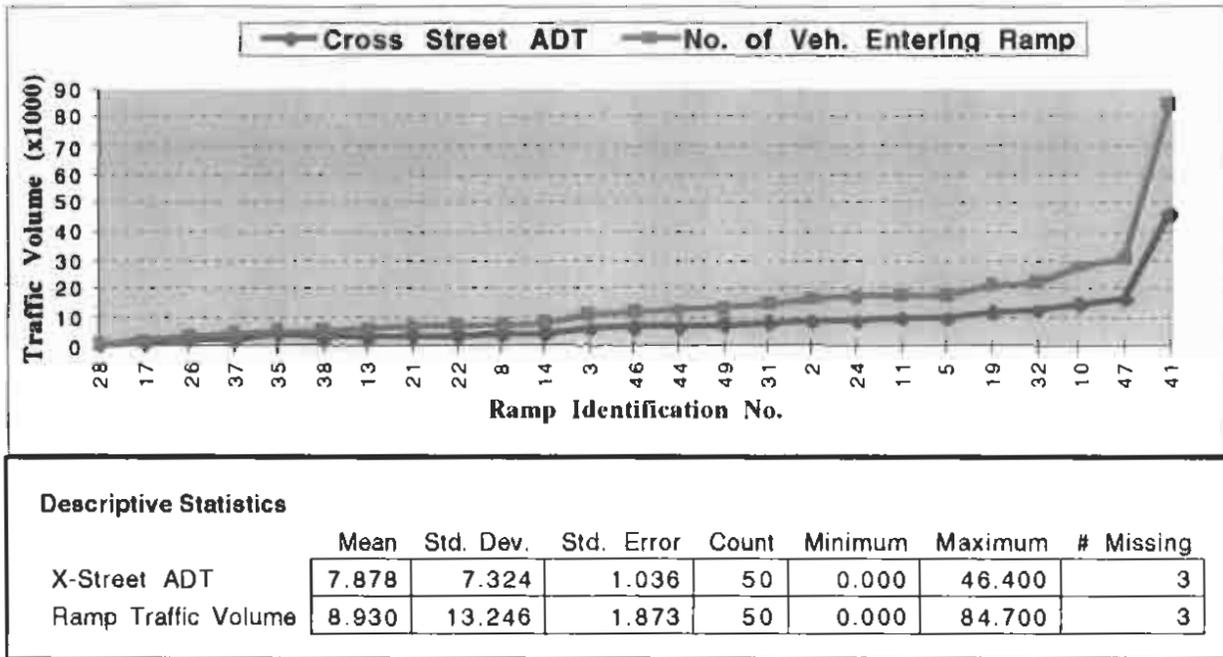
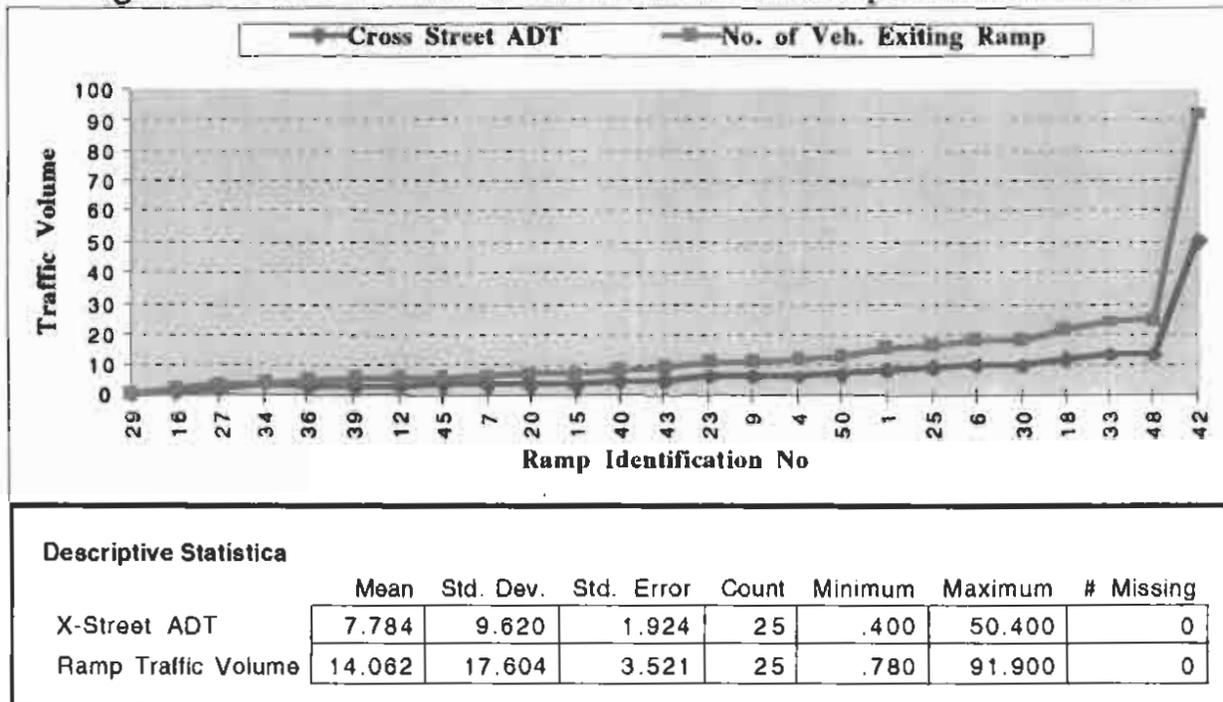


Figure 25 Traffic Volume on Half Diamond On-Ramps and Cross Street



Discussion of results on half diamond ramps

Figure 20 through 25 along with their descriptive statistics are self explanatory. When considering a half diamond ramp safety performance, both accident frequency and accident rate along with the traffic volume on ramp and cross street should be taken into account with engineering judgment. Notice that there is a great deal of variability both in accident frequency and accident rate for the sample of ramps.

As a general rule, a ramp with accident or accident rate greater than mean plus two standard deviation is said to have significantly higher accident or accident rate (with 95 percent confidence level). For example, ramp number 10, and 32 in figure 20 has fatal + injury accident frequencies which are significantly higher than the average half diamond off-ramp accident frequency. The same two ramps show higher than average accident rate in figure 21. However, their accident rates increase above the average is not statistically significant. Therefore, the significant accident frequency on these ramps can be attributed to higher ramp volume as suggested in figure 24.

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